UV DISINFECTION COST STUDY

Cost Study Report

FOR

METROPOLITAN WATER RECLAMATION DISTRICT OF GREATER CHICAGO VOLUME 1 OF 2

NORTH SIDE WATER RECLAMATION PLANT

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EXECUTIVE SUMMARY

Introduction

The Technical Memorandum 1WQ Disinfection Evaluation (TM1-WQ) was completed in August 2005 for the Metropolitan Water Reclamation District of Greater Chicago (MWRDGC or District) as part of the comprehensive Infrastructure and Process Needs Feasibility Study (Master Plan) for the North Side Water Reclamation Plant (NSWRP) and a Water Quality (WQ) Strategy for affected Chicago Area Waterways. The TM1-WQ reviewed the alternative disinfection technologies available for use at the District's North Side, Calumet and Stickney Water Reclamation Plants and provided an initial estimate of possible construction cost for the facilities. On the basis of that report the District requested further investigation into UV disinfection. The findings of the Preliminary Cost Opinion for Ultraviolet (UV) Disinfection Facilities Study at the North Side Water Reclamation Plant are presented in this Report.

Objectives

This evaluation is based upon the TM1-WQ, the comments received from the USEPA as part of the Use Attainability Analysis (UAA) evaluations, and new information obtained since the previous work. The primary objectives of the evaluation presented in this report are:

- To describe the conceptual facilities developed as part of this study including their basis of design and the assumptions used for their development
- To develop a Level 3 Preliminary Opinion of Probable Construction Cost per the Association for the Advancement of Cost Engineering recommended practices for the proposed facilities at NSWRP
- To develop annual maintenance and operations (M&O) costs for the facilities
- To use the costs developed for NSWRP UV Disinfection Facilities to project the costs for similar facilities at the Calumet Water Reclamation Plant (CWRP)

Proposed Facilities

The study reviewed the proposed facilities for the UV Disinfection Alternative included in TM-1WQ including the four primary components: site work, a low lift pump station, tertiary filters, and UV disinfection. Through that review, it was determined that the low lift pump station and the tertiary filters required re-evaluation.

At the time TM-1WQ was developed, very little information was available regarding the water quality of the plant effluent as it related to ultraviolet light transmissivity, and the that data was available indicated low transmissivity levels. Because of the conceptual nature of TM-1WQ, tertiary filters were included in initial proposed facilities in order to improve disinfection effectiveness by removing water components that would inhibit the disinfection process, although costs were also provided without tertiary filters. Since that time, additional water quality data has been collected by the District and review of that data indicates that UV transmissivity is within the minimum range necessary for UV disinfection without filtration. Therefore, tertiary filters are not included in the proposed facilities presented in this report. However, the exclusion of tertiary filters from this report should not suggest that tertiary filters would not be required in the future to meet stricter suspended solids or phosphorous limits, or that tertiary filters would not improve the effectiveness of a UV disinfection process. As concluded in the NSWRP Master Plan, space would be reserved on the site for future tertiary filter facilities.

Because tertiary filters would not be required to be added as part of the implementation of UV disinfection, the need for a low lift pump station was questioned. Additional pumping would be required only if the head losses added by the UV Disinfection Facilities and associated flow conduits and flow splitting structures exceed the available head at the plant. To determine the additional head losses, a hydraulics evaluation was performed.

Hydraulics

The hydraulic model developed for the Master Plan was modified to include the additional effluent conduits, gate structures, and UV channels/reactors. The model was used to determine the actual head losses expected following implementation of the UV Disinfection Facilities. The results of this evaluation showed conclusively that projected head loss through the plant exceeded the available head at the plant by over 1.5 feet and, therefore, identified the need for a Low Lift Pump Station (LLPS) in order to treat peak flows at the 100-year flood elevation once the UV disinfection system was installed.

Disinfection Technology

The Trojan UV4000™Plus system was used to develop the basis of design for the UV disinfection system at the NSWRP due to the lower number of lamps required compared to other systems and the recommendations of a team of disinfection experts that evaluated the available technologies during the Master Plan effort. During this study, the details of the implementation of this UV technology were updated by consultation with the manufacturer and incorporated into the basis of design. In addition, a phone survey of other facilities of similar size and source water quality was conducted. This survey revealed several important conclusions including the following:

- When using ferric salt addition for improved settleability of solids in the treatment process upstream of UV disinfection (similar to the NSWRP Master Plan's recommendation for future phosphorous removal), an increase in the fouling rate was experienced.
- The level of maintenance and operations efforts was highly variable and site specific, even with plants using the same technology and source water.
- The most effective method of power control for the UV system is highly site specific and has a great impact on the disinfection effectiveness and the energy effectiveness of the system.

Due to the size of the proposed NSWRP UV Disinfection Facilities, which would be among the largest continually-operating UV disinfection systems in the world, CTE recommends the District undertake an extensive design program which includes review of system specific independent validation studies, collimated beam testing, UV transmittance testing, and a reasonably sized pilot facility. This program would determine, among other factors, the following information in-situ:

- Appropriate control sequences and optimization for the UV disinfection equipment, including appropriate sensing equipment to allow advanced power management
- In-situ disinfection performance including fouling rates or the lamps with and without ferric salt addition
- Actual M&O requirements in terms of labor and consumables as well as space requirements to complete required maintenance activities

Site Constraints

As part of the study, a proposed layout of the disinfection facilities at NSWRP was developed including the Low Lift Pump Station, UV Disinfection Facilities, related gate structures/effluent conduits, and space reserved for future tertiary filters (See **Figure ES-1** and Volume 2 of this report). Due to existing constraints of the site, several significant civil improvements would be required. At the NSWRP, the proposed facilities would extend into the existing CTA railway embankment and therefore, temporary sheeting and a permanent retaining wall would be required to support the railway embankment. These works would need to be coordinated with the CTA to minimize disruption. In addition, due to existing geotechnical conditions in the area available for the UV Disinfection Facilities, deep foundations would be required for the proposed structures. These required civil improvements add significant complexity and cost to the proposed project.

Preliminary Cost Opinion

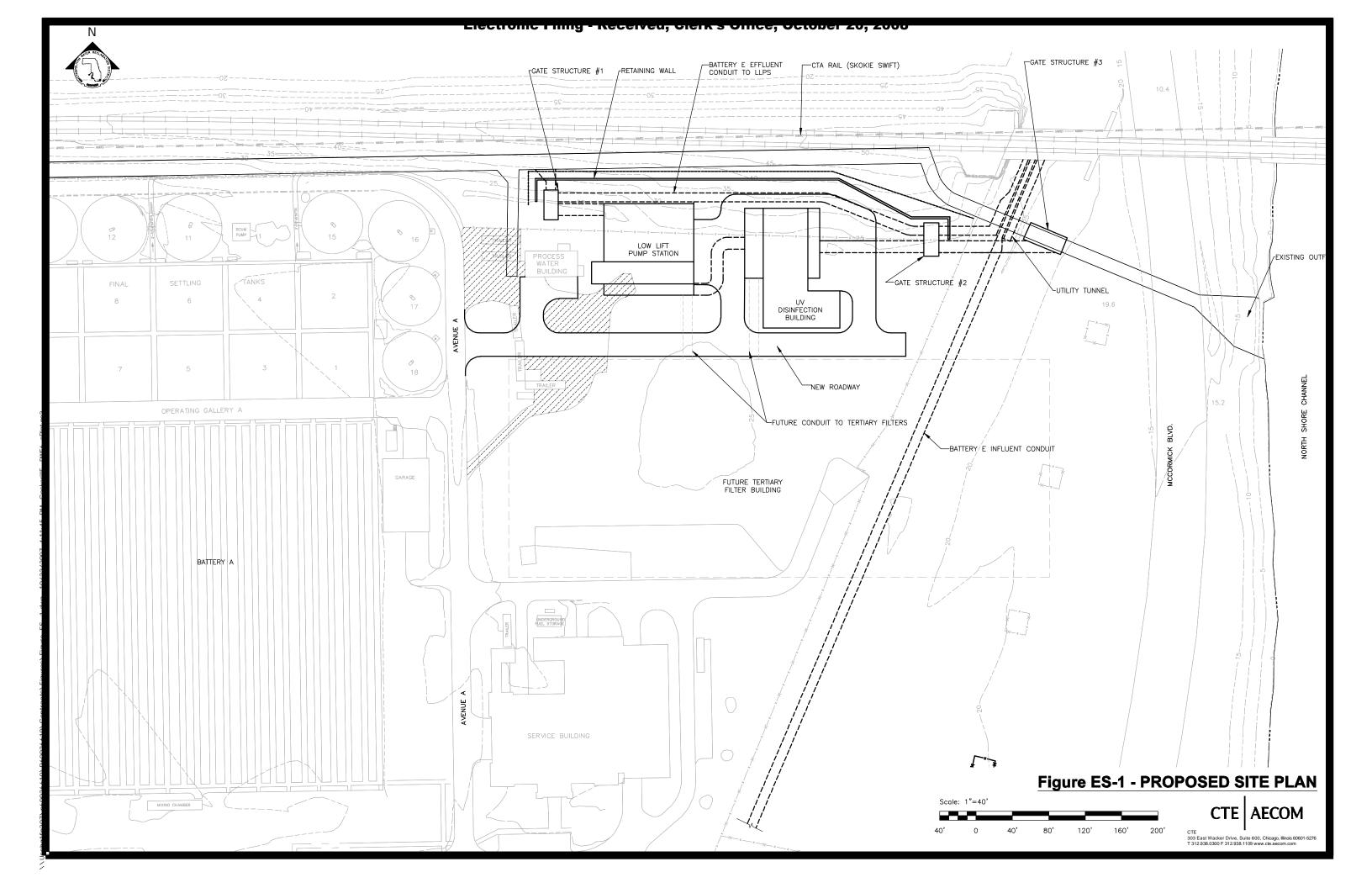
The preliminary opinion of probable construction cost (OPCC) for NSWRP and CWRP UV Disinfection Facilities is shown in **Table ES-1** below. As shown, the projected construction cost for the NSWRP UV Disinfection facilities is \$108.8 million and the projected construction cost for the CWRP UV Disinfection facilities is \$109.4 million.

Due to the essential similarities between the proposed disinfection facilities for the two sites, the OPCC was developed for NSWRP and then adjusted for the CWRP. To estimate the costs for the UV Disinfection Facilities at CWRP, CTE deducted the costs for the deep foundations required at NSWRP, multiplied the remaining capital cost estimate by the ratio of 480 MGD to 450 MGD, and added the cost for demolishing the existing chlorine contact chambers. The details of the basis of design for the proposed facilities and the methods of developing the OPCC are presented in the body of this report.

Table ES-1 – NSWRP & CWRP UV Disinfection Facilities Preliminary OPCC and M&O Costs

Capital Cost Estimates					
	North Side WRP	Calumet WRP	Total		
NSWRP UV Pilot Plant	\$2,200,000	-	\$2,200,000		
ComEd Service Upgrade	\$2,900,000	\$130,000	\$3,030,000		
A. General Sitework	\$27,200,000	\$27,800,000	\$55,000,000		
B. Low Lift Pump Station	\$27,000,000	\$28,800,000	\$55,800,000		
C. Disinfection System	\$49,500,000	\$52,800,000	\$102,300,000		
Total Capital Cost	\$108,800,000 \$109,530,000		\$218,330,000		
Maintenance & Operations Cost Estimates					
A. General Sitework	\$130,000/yr	\$130,000/yr	\$260,000/yr		
B. Low Lift Pump Station	\$1,100,000/yr	\$890,000/yr	\$1,980,000/yr		
C. Disinfection System	\$3,590,000/yr	\$3,490,000/yr	\$7,090,000/yr		
Total Annual M&O Cost	\$4,830,000/yr	\$4,520,000/yr	\$9,330,000/yr		
Total Present Worth M&O Cost	\$111,900,000	\$104,600,000	\$216,000,000		
Total Present Worth	\$220,700,000	\$214,100,000	\$434,800,000		

All costs in 2007 dollars.



Electronic Filing - Received, Clerk's Office, October 20, 2008 EXISTING CHLORINE BUILDING TO BE DEMOLISHED EXISTING EFFLUENT CHAMBER TO BE REUSED EXISTING INFLUENT CHAMBER TO BE REUSED.— EXISTING CHLORINE CONTACT TANKS TO BE DEMOLISHED (NOT SHOWN) - LLPS SLUDGE CONCENTRATION TANK BLDG, -UV BUILDING -NEW PAVEMENT -NEW EFFLUENT CONDUIT (TYP) CALUMET DRIVE RESERVED FOR FUTURE TERTIARY FILTERS **Figure ES-2 - PROPOSED SITE PLAN CALUMET WRP** Scale: 1"=60' CTE | AECOM

240' 300'

CTE 303 East Wacker Drive, Suite 600, Chicago, Illinois 60601-5276 T 312.938.0300 F 312.938.1109 www.cte.aecom.com

180'

1.0 INTRODUCTION

1.1 Background

This report has been developed to present the findings of the Preliminary Cost Opinion for Ultraviolet (UV) Disinfection Facilities Study at the Metropolitan Water Reclamation District of Greater Chicago's (MWRDGC, or District) North Side Water Reclamation Plant (NSWRP) in Skokie, Illinois. This memorandum continues the work began in TM1-WQ, which was developed previously as part of the comprehensive Infrastructure and Process Needs Feasibility Study (Master Plan) for the NSWRP and a Water Quality (WQ) Strategy for affected Chicago Area Waterways.

The TM1-WQ documented the results of a CTE study of effluent disinfection alternatives for the District's North Side, Calumet and Stickney WRPs. In that study, a task force of national experts (referred to as the Blue Ribbon Panel) reviewed available disinfection technologies and their range of pathogen destruction efficiency, disinfection byproducts and impacts upon aquatic life and human health. Their investigation also included an examination of the environmental and human health impacts of the energy required for the operation of the facility and for the processing and production of process chemicals. Based on economic and non-economic evaluation of alternatives, ozone disinfection and UV disinfection were selected and preliminary design and cost estimates were developed. Based on the results of that subsequent evaluation, the District has determined that UV disinfection is the most cost-effective alternative.

1.2 Objective

Per the District's request, further evaluation of the UV disinfection technology is required. This additional evaluation is based on the TM-1WQ, the comments received from the United States Environmental Protection Agency (USEPA) as part of the Illinois Environmental Protection Agency's (IEPA) Use Attainability Analysis (UAA) evaluations, and new information obtained since the previous work. The primary objectives of the evaluation presented in this report are:

- To describe the conceptual facilities developed as part of this study including their basis of design and the assumptions used for their development
- To develop a Level 3 (per the Association for the Advancement of Cost Engineering) Preliminary Opinion of Probable Construction Cost for the proposed facilities at NSWRP and Calumet WRP
- To develop annual maintenance and operations (M&O) costs for the facilities
- To use the costs developed for the NSWRP UV Disinfection Facilities to project the costs for similar facilities at the Calumet Water Reclamation Plant (CWRP)

1.3 General Design Standards

Where applicable, the latest version of the codes and standards from the following institutions/organizations would govern the design:

- 1. State of Illinois, Illinois Recommended Standards for Sewage Works, Title 35.C.II.370.
- Great Lakes Upper Mississippi River Board of State and Provincial Public Health and Environmental Managers, Recommended Standards for Wastewater Facilities (Ten States Standards)

- 3. National Fire Protection Association Standard 820 Standard for Fire Protection in Wastewater Treatment and Collection Facilities
- 4. International Building Code, 2003
- 5. Metropolitan Water Reclamation District of Greater Chicago Standard Specifications

1.4 Organization of this Report

The Cost Study Report is divided into two volumes. Volume 1 is the text and backup materials presenting the findings of the additional evaluation of the cost of implementation of UV disinfection at the North Side WRP and Calumet WRP. Volume 2 is the conceptual level drawings presenting the preliminary layout and some details of the proposed facilities from which the preliminary opinion of construction cost was developed.

The basis of this evaluation is the proposed facilities necessary for UV Disinfection Facilities and related ancillary improvements at the NSWRP. The sections of Volume 1 are organized as follows:

Section 2 – Discussion of the hydraulic analysis that was performed based on updated information and that forms the basis of decisions regarding the need for a low lift pump station and the general layout of the facilities.

Sections 3 through 8 – Discussion of the basis of design for the proposed facilities by design discipline and the assumptions necessary for development of the conceptual design presented in Volume 2.

Section 9 – Discussion of areas that require further analysis during the preliminary design of the proposed facilities due either to their critical nature regarding design decisions or their large impact on potential construction or operating costs.

Section 10 - Summary of the Preliminary Opinion of Probable Construction Cost (OPCC) and annual operating costs as well as discussion of the assumptions used to develop those costs.

Section 11 – Presents the projected schedule of implementation of the proposed facilities if the decision to proceed is made in the future.

The final section – **Section 12** – projects the capital and operating costs for implementation of identical facilities at the Calumet WRP. Due to their similar size, it was determined that the detailed evaluation of the costs for implementation at North Side WRP could be used for development of costs for the Calumet WRP. To develop this estimate, the costs estimated for NSWRP were adjusted for site specific costs at each site and multiplied by the ratio of peak design capacities at the two plants. Section 12 details these adjustments and presents the summary of the costs at CWRP.

2.0 HYDRAULICS

2.1 Recommended Alternative from Disinfection Cost Study Hydraulic Evaluation

Various disinfection layout alternatives were considered in the Disinfection Cost Study Hydraulic Evaluation. All alternatives included a Low Lift Pump Station (LLPS) and UV Disinfection Building (UV). A discussion of the need for the LLPS is included later in this section. For a full analysis of the alternatives considered and the evaluation process, see Appendix A. The recommended alternative minimizes the number of pumping facilities required and is the most easily modified to accommodate the possible future addition of tertiary filters if required for more stringent effluent limitations on suspended solids or phosphorous. A schematic for the recommended alternative is included as **Figure 2.1-1.**

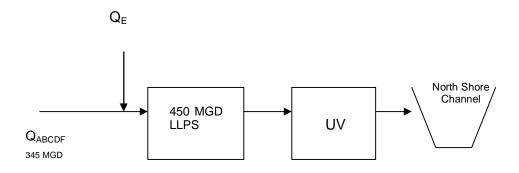


Figure 2.1-1 – Recommended UV Disinfection Facilities Arrangement

2.2 Hydraulic Analysis of the UV Disinfection Facilities

2.2.1 Objectives

For this study, modifications were made to the preliminary hydraulic model created under the Master Plan in order to provide a more detailed hydraulic analysis of the UV Disinfection Facilities. These modifications included adding the additional effluent conduits, gate structures, UV channels and reactors, and Low Lift Pump Station to provide a more comprehensive hydraulic evaluation of the UV disinfection facilities.

2.2.2 Overview

The hydraulic analysis was completed using a spreadsheet utilizing standard open channel and closed conduit flow equations to represent the NSWRP. The hydraulics evaluated were for the year 2040 conditions, including both infrastructure and permit-related improvements. A peak flow of 450 mgd was used. Flow in excess of 450 mgd is diverted to the TARP system. Return activated sludge flows were added to the influent where appropriate. In order to reflect the nutrient removal processes, internal mixed liquor recycled flows were used in the hydraulic analysis of the activated sludge aeration tanks.

Similar to the analysis performed previously as part of TM1-WQ, critical flow paths were identified as those which would result in the greatest total headloss through the facility. Other flow paths through the facility experience lower headloss and, as such, further

evaluation of these flow paths is not warranted because changes along these flow paths will not change the results of the evaluation. These critical flow paths were modeled from the North Shore Channel Outfall to immediately upstream of the coarse bar screens in the Pump and Blower House. The two flow paths identified as critical flow paths for this study are as follows:

- 1. Critical flow path through Battery A
- 2. Critical flow path through Battery E

2.3 Assumptions

Due to the preliminary nature of the selected site plan, assumptions were made in the development of the hydraulic model. These assumptions are as follows:

- 1. All NSWRP drawings obtained from MWRDGC are on the same datum, known as the Chicago City Datum (CCD).
- 2. The CCD has not changed since the plant was originally constructed in the 1920's.
- 3. Flow through future Battery E is 105 MGD and it is treated as a base loaded plant. Flow through existing Batteries A, B, C, D, and F is the remainder and will be 345 MGD at peak flow. Flow over 450 MGD is diverted to TARP.
- 4. Return flow from the Grit Dewatering System and Scum Concentration Tanks as well as supernatant from the Sludge Concentration Tanks are negligible.
- 5. Flow reduction as a result of primary sludge removal is negligible.
- 6. The 100-year flood elevation the North Shore Channel is 12.30 CCD, as calculated in the Chicago Canal System Model, UNET. Appendix A provides selected pages from the USACE's Chicago Underflow Plan (CUP) Design Report presenting these results. Pre-Stage 1 (Stage 1 of McCook Reservoir Construction) values are used since the USACE's current estimate for completion of Stage 1 construction is 2020 or later.
- 7. Hydraulics through the existing Meter Building will control flow splits among Battery A, B, C, D, and future F proportional to the battery volumes.
- 8. Flow splits evenly based on aeration tank volume within each battery.
- 9. Flow splits evenly among the aerated grit channels located in the Grit Building.
- 10. Return Activated Sludge (RAS) flows were calculated to be 55% of total influent flow.
- 11. Internal recycle flow for total nitrogen removal was calculated to be 150% of total influent flow per battery.
- 12. Baffle walls (for TN removal) were assumed to be mounted where mixed liquor flows from underneath one baffle wall to the top of the next baffle wall, creating a "up and down" flow pattern.
- 13. The longest flow path through each treatment process was used.
- 14. Tank geometry downstream of the aeration tank effluent weirs (Operating Gallery and Final Settling Tanks) in Battery A was assumed to be similar to that of existing Battery D.
- 15. Geometry of Batteries E and F were assumed to be similar to that of existing Battery D.
- 16. Proposed primary settling tank geometry was assumed to be similar to that of the existing circular primary settling tanks.
- 17. Velocity in Disinfection Influent and Effluent Distribution Chamber is zero

- 18. Battery E is to be pumped via the proposed low-lift pump station on the existing (southern) NSWRP site.
- 19. Battery E influent flows by gravity from downstream of the Grit Building to the north site resulting in the facilities being lower in elevation than the same facilities on the existing site.
- 20. Disinfection channel effluent weir gate is assumed to be downstream water surface elevation (WSE) + 0.5'.
- 21. Tertiary Filters Are excluded from the model, but the LLPS pumps can be modified to accommodate the additional head associated with this process.
- 22. The following modeling equations were used:
- a. Pressure Flow Hazen Williams Equation
- b. Open-Channel Flow Manning's Equation
- c. Flow junctions Pressure Momentum Analysis
- d. Hydraulic coefficients used in developing this model include:
 - 1. Hazen Williams, C 110 (concrete)
 - 2. Manning's, n
 - i. Regular channel 0.013
 - ii. Aerated channel 0.035

2.4 Results

After calculation of head losses through the plant by evaluating each existing and proposed unit process, the preliminary hydraulic analysis shows that over 15.5 feet of headloss is required to convey flow through the NSWRP given the existing facilities in Battery A and the proposed facilities in Battery E. Only 14.54 feet of head is available to convey the flow entirely by gravity through the same flow paths.

Table 2.4-1 presents the total headloss through various portions of the plant for Battery A and Battery E for comparison. Tertiary filters are not included in the hydraulic analysis. The hydraulic profiles show the estimated WSEs at the maximum flow of 450 mgd. Flow that exceeds 450 mgd is diverted into the TARP system.

Table 2.4-1 – Summary of Headloss through the Unit Processes at NSWRP (Proposed)

Process/Flow Area	Battery A	Battery E
Pump and Blower House Discharge to Aerated	2.03	2.03
Grit Discharge Chamber		
Aerated Grit Discharge Chamber to PSTs	1.03	2.39
Primary Settling Tanks	1.83	2.44
Aeration Basins and Final Settling Tanks	5.98	2.72
Effluent Conduit to Low Lift Pump Station Wet	0.67	1.96
Well	0.07	1.00
LLPS Discharge to UV Disinfection Effluent	3.36	3.36
Chamber	5.50	3.30
UV Disinfection Effluent Chamber to Outfall	.66	.66
Total	15.56	15.56

Notes: Values in feet of headloss.

Does not include head dissipated due to minimum pump head requirements.

Table 2.4-2 presents the final water surface elevations (WSE's) through the plant including the Low Lift Pump Station (LLPS) and UV Disinfection Building. Note that the WSE provided hear take into account the headloss summarized in Table 2.4-1 above as well as including the LLPS and the head gain that is provided by that facility. Due to the

constraints of the pumping equipment, the head gain is greater than the minimum required to convey the flow through the remainder of the process train.

Table 2.4-2 – Summary of Proposed WSE including UV Disinfection Facilities

Location	Combined	Battery A	Battery E
North Shore Channel 100-yr Flood			
Elevation	12.30		
D/S WSE @ New Surge Chamber*	12.96		
U/S WSE @ New Surge Chamber*	15.96		
WSE @ Disinfection Effl Channel	16.52		
WSE just U/S of Weir Gate	18.03		
WSE just D/S UV Reactor	18.08		
WSE just U/S UV Reactor	18.83		
WSE just D/S of influent gate	18.87		
WSE in LLPS Discharge Channel	19.88		
LLPS Wet Well High Water Level (HWL)	16.00		
Final Settling Tank Effluent Chambers		16.67	17.96
Aeration Tank Effluent Chambers		20.39	18.88
Aeration Tanks		20.69	19.62
Primary Tank Effluent Chambers		22.65	20.68
Grit Building Effluent Chamber	25.51		
U/S of Fine Screens	25.76		
Aerated Grit Tank Influent Chamber	26.51		
Siphon Room	27.54		

Notes: All WSE in CCD.

* Includes approximately 2.5 of head dissipated due to min pumping requirements.

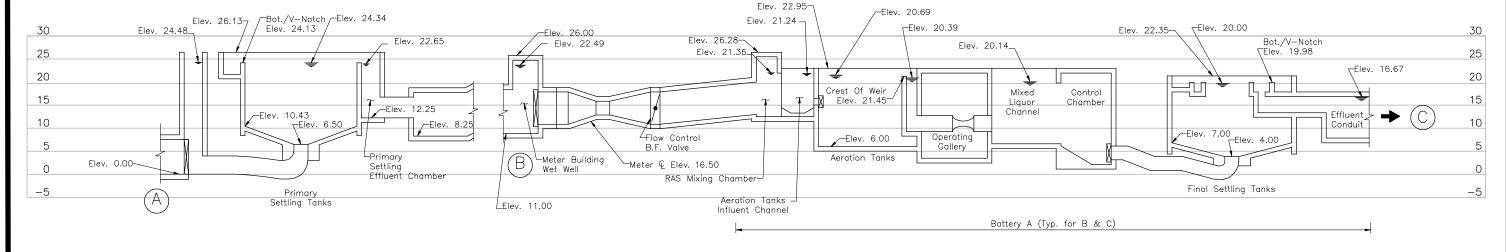
WSE - Water Surface Elevation

D/S – Downstream U/S – Upstream

Figure 2.4-1 and 2.4-2, on the following pages, contain hydraulic profiles of the two critical flow paths with the UV disinfection facilities and the available freeboard at the locations where water surface elevations (WSEs) were calculated at the maximum day flow.

2.5 Conclusion

Based on the preliminary hydraulic analysis performed as part of this study, the total head required to convey flow through the Northside WRP with the proposed UV disinfection process is 15.56 feet at the peak flow rate of 450 mgd. From the effluent channels of Batteries A and E, 3.36 feet of total head is required to convey the flow to the surge chamber. The pumping station is required to provide approximately 1 foot of head to convey the flow through the UV disinfection process. Due to the minimum discharge pressure requirements of the pumping equipment, approximately 2.88 feet of head is actually provided. The excess head would be dissipated at the surge chamber.

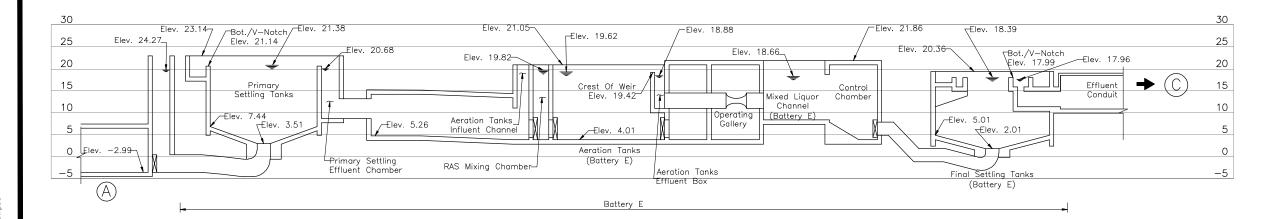


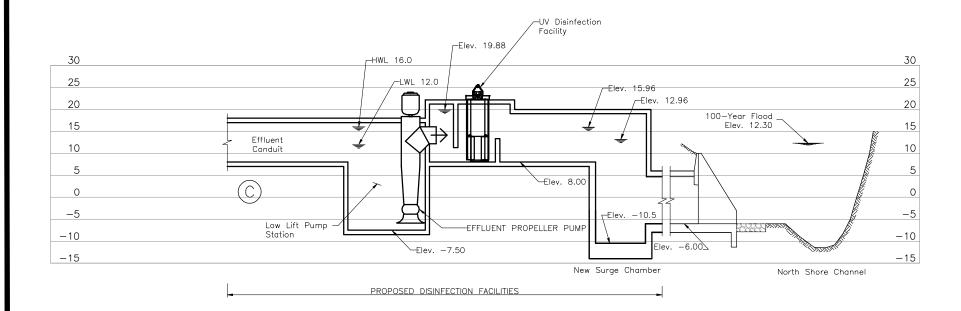
<u>Legend</u>

- (A) Flow diversion from Headworks to Batteries on Existing Site and North Site Battery E. (Existing Site 345 mgd, Battery E 105 mgd)
- (B) Flow split at Meter Building Wet Well to Batteries A & F

FIGURE 2.4-1 HYDRAULIC PROFILE FOR BATTERIES A-F AFTER IMPLEMENTATION OF UV DISINFECTION FACILITIES







Legend

- (A) Flow diversion from Headworks to Battery E [Q (Battery E) = 105 mgd]
- © Flow junction from Batteries A, E, & F. (Total Q = 450 mgd)

FIGURE 2.4-2 HYDRAULIC PROFILE FOR BATTERIES E AFTER IMPLEMENTATION OF UV DISINFECTION FACILITIES



3.0 NSWRP DISINFECTION PROCESS

3.1 Introduction

The District has preliminarily selected the medium-pressure high-intensity (MP-HI) UV disinfection technology for disinfection of final effluent at the NSWRP. This section presents the results of further evaluation of the MP-HI UV disinfection technology per the District's requirement. In the following discussion, the basis of design of the MP-HI UV system is presented and a preliminary basis of design of the UV system to be used at the NSWRP is provided. The low-lift pump station's basis of design, operation and layout are provided later in this section.

3.2 UV Disinfection System

3.2.1 Background

The Technical Memorandum on the UV Disinfection Technology, included in Appendix B, incorporates the following:

- Information from literature including technical proceedings from the Water Environment Federation (WEF), Water Environment Research Foundation (WERF), proceedings from the latest Disinfection conference series undertaken by WEF, American Water Works Association (AWWA), and International Water Association (IWA). This information provided the latest updates in the UV disinfection technology.
- Updated recommendations on the UV system from four manufacturers Trojan Technologies, Aquionics, Calgon Carbon, and Severn Trent Services (STS)/Quay.
- Reference information on experience of UV disinfection at five selected facilities

 Racine WWTP (Racine, WI), R.L. Sutton WRF (Cobb County, GA), Grand Rapids WWTP (Grand Rapids, MI), Jacksonville WWTP (Buckman, FL), and Valley Creek WWTP (Valley Creek, AL). A summary of the information collected through the phone survey is provided in Appendix B, and important inferences from the phone survey are as follows.
 - 1. Fouling due to iron in the effluent has been a problem at the Racine, Sutton, and Grand Rapids facilities. Fouling results in lower then expected disinfection performance, higher operating costs, and higher M&O efforts. The iron in the effluent at all three plants was primarily from the chemical phosphorus removal using Ferric Chloride. At Grand Rapids WWTP, the chemical addition is upstream of the secondary treatment process; staining of sleeves was found only when the chemical addition was in the secondary clarifiers. At the Sutton WRF, fouling of lamps due to iron is observed although chemical addition is upstream of secondary process and sand filters are used upstream of the UV disinfection system. At the Racine WWTP, fouling may be due to ferric chloride addition and/or due to the additional iron brought by the ferric sludge from another water treatment plant, although operational controls are used to prevent both sources from occurring simultaneously.

- 2. Calcium fouling due to hardness in the source water is not a significant problem because of the automatic mechanical/chemical cleaning system that dissolves and wipes away any scales. The lack of calcium hardness was observed in all five plants including the Racine and Grand Rapids utilities which have Lake Michigan source water and is attributed to the automatic cleaning system performance.
- 3. The frequency of cleaning and changing of the cleaning solution is specific to the utility and would have to be determined only by experience.
- 4. Labor requirements varied amongst facilities, with some facilities requiring more labor to handle the fouling caused by iron salt addition.
- As long as other processes in the plant are performing as desired, all five facilities were satisfied with the UV disinfection system because it met their disinfection goals.

In conclusion, the phone survey had revealed that fouling of the quartz sleeves is a concern for this application, particularly if iron salts are added for phosphorous removal in the future. In addition, the phone survey results suggest that the manufacturer's recommended labor assumptions for routine maintenance including cleaning and inspection of the lamps is too low for this application. Using this information and the updated information available from manufacturers, a preliminary basis of design of the MP-HI UV disinfection system has been developed for disinfection of the final effluent at the NSWRP.

3.2.2 Basis of Design

The MP-HI system involves sending the secondary or tertiary effluent through channels containing banks of MP-HI UV lamps. Refer to the process drawings included in Volume 2 of this report. The Trojan UV4000™Plus system is used here to develop the basis of design for the UV disinfection system. The system consists of a power supply, an electrical system, a reactor, MP-HI lamps, a mechanical and chemical cleaning system, and a control system. The MP-HI UV lamps are enclosed in individual quartz sleeves for protection against dirt and breakage. Reactor chambers (open channels) hold the lamps in a horizontal configuration. The effluent weirs and level sensors are used to keep the lamps submerged under the effluent water. This submergence ensures that the lamps do not overheat, thereby preventing lamp life reduction or burnout.

The UV system is assumed to operate from March to November each year. During the winter months, the equipment would sit idle as the flow is bypassed around the LLPS and UV Disinfection Building. However, due to the size of the facility including five reactors and over 1600 lamps, maintenance activities would be conducted every working day from March to November and periodically during the winter months. It is reasonable to expect that the area would continue to experience normal weather patterns for the Chicago area including extreme weather during all four seasons. In order to protect the safety of the M&O staff, ensure operational and maintenance-related productivity, and protect the UV equipment from adverse weather common to the Chicago area including high winds, rain, lightning, snow, and extreme temperatures, the UV system would be enclosed in a building.

3.2.2.1 Influent Characteristics

The water quality characteristics that affect UV transmittance include iron, hardness, suspended solids, humic materials and organic dyes. These effluent constituents have a tendency to absorb UV light and thus impact the disinfection process. The UV transmittance generally needs to be above 65% for effective disinfection. The water quality testing done at the NSWRP and CWRP as part of the UV disinfection technology trials conducted by the District during 2006-2007 showed an average transmittance above this minimum value. Refer to Appendix B for more information regarding the influent characteristic testing. The total suspended solids limit is projected to be 15 mg/L for the purposes of sizing the UV system.

3.2.2.2 Reactor Configuration and Hydraulics

An open channel is used as a reactor. Each channel has one reactor with two banks each. Each bank includes stainless steel UV modules with the MP-HI lamps mounted on them and arranged in a linear configuration to increase intensity along the linear axis by avoiding UV emission losses due to self absorption, reflection or refraction that can occur if a UV lamp were twisted into loops or spirals. The lamps are positioned horizontally and parallel to the flow.

The optimum hydraulic scenario for this system involves turbulent flow with mixing while minimizing head loss. Reactor design, including inlet and outlet flow distribution is done so that the unit operates close to a plug flow. Inlet conditions are designed to distribute the flow and equalize velocities. Sufficient length is provided in the channel upstream of the reactor to allow equalization of the flow. A motorized weir gate is provided downstream of each reactor to control the water level at a constant level with little fluctuation within the UV disinfection reactor.

3.2.2.3 Lamps and UV Intensity Control

The MP-HI lamps produce polychromatic radiation, which is concentrated at select peaks throughout the germicidal wavelength region. The IEPA requires a minimum UV dose of 40 mW-s/cm² that was considered during the design of the UV system. It may be possible to document a lower required dose to the regulating body (IEPA) during design development, but lacking such data, this study does not deviate from the required minimum dose.

Each lamp is enclosed in a quartz sleeve because quartz effectively protects the lamps while minimizing any UV transmission losses. Electronic ballast for each lamp is used to control the power to the lamp. If the UV dose is to be reduced, the variable output electronic ballast regulates the power to the lamp from 100% to 30%. Entire banks can also be turned off if there is no flow. This allows dose-pacing based on the secondary or tertiary effluent flow and quality, which helps save power and lamp life and hence reduce costs.

3.2.2.4 Lamp Fouling and Cleaning

The MP-HI lamps operate at a temperature range of 600 to 900 degree C. These warm temperatures promote fouling on the surface of the quartz sleeves when the lamps are placed directly within the wastewater stream. Iron is the most abundant metal in these scales along with other mineral salts and oil, grease, suspended solids deposits, and biofilms. If no tertiary treatment is provided, physical debris may contribute to fouling as well.

Since lamp fouling significantly reduces the effectiveness of UV disinfection by blocking the UV rays, calculation of the UV dose incorporates a term called the "fouling factor", which allows the designer to estimate the effects of fouling on performance of the disinfection process. To combat fouling, a chemical and mechanical cleaning system is proposed for the MP-HI UV disinfection system. The latest technology uses a system of mechanical wipers and sleeves containing cleaning chemicals surrounding the lamp. The cleaning solution contains some acidic solution that prevents fouling. This cleaning system can be programmed to clean at a set frequency without the need for disrupting the disinfection process. The cleaning solution needs to be replaced periodically depending on the type of solution used and characteristics of the effluent water quality. Similar facilities using Lake Michigan as source water have found that changing the cleaning solution on a monthly basis is required for adequate performance.

Due to the mechanical and chemical features of the Trojan automatic cleaning system, the IEPA accepts the default value of 100% for the fouling factor in the UV_{dis} software package (dosage modeling software) for sizing the equipment. Based on the phone survey results that indicated a higher potential for fouling in the event of Lake Michigan source water with ferric salt addition, the District has elected to incorporate a safety factor of 10% by using a fouling factor of 90%.

3.2.3 Process Control

An automated process control must be provided to facilitate online pacing of the UV dose to prevent overdosing that wastes electricity and to avoid under-dosing that would not meet the disinfection regulatory requirements and goals. The process control should also allow the dose-pacing to be interfaced with the plant's overall supervisory control and data acquisition (SCADA) system. The flow, lamp output, and water conditions are measured in pacing of the dose, and an algorithm is developed based on long-term measurements to predict necessary system adjustments, maintenance, and component replacements.

Programmable logic control (PLC) technology must be used for dose pacing in the MP-HI UV disinfection system. The PLC interacts with the ballasts, sensors, and online monitoring technology for each disinfection unit. The PLC then interacts with the plant's overall control system to allow remote monitoring and adjustment of the system. The PLC should be supplied by the manufacturer of the unit.

3.2.4 Safety

The high voltage power supplies for the MP-HI UV disinfection system may pose an issue as the lamps are submerged in the water most of the time and compliance with electrical safety codes is required. In addition, UV light poses a risk to personnel and can cause damage to skin or eyes upon exposure. Submerging a lamp in water, even if it is just a few inches below the surface, greatly reduces the intensity. During operation the system should be covered by hatches and should be designed to ensure constant water levels to minimize the risk of UV exposure.

3.2.5 Proposed Design Criteria for UV Disinfection Equipment

Based on a review of the information provided by the UV equipment manufacturers and the experience of five other facilities (Appendix B), it is observed that Trojan Technologies provides a widely-used low-maintenance solution for final effluent disinfection. The design of the MP-HI UV disinfection system for the North Side WRP is

based on the Trojan UV4000™Plus equipment provided by Trojan Technologies. The basis of design is given in **Table 3.2-1**.

Table 3.2-1 - Design Parameters for UV Disinfection Unit at NSWRP

A 1: 1111 A 111				
Capacity and Water Quality				
Design flow, mgd	450			
Average flow, mgd	333			
Maximum TSS ^a , mg/L	15			
Pre-Disinfection Effluent E.Coli Count ^b , cfu/100 mL, maximum	200,000			
(Assumed)				
Post-Disinfection Effluent E.Coli Count Target ^c , cfu/100 mL	1030			
Effluent hardness ^d , mg/L as CaCO ₃	270			
Dosage				
UV transmittance, minimum, %	65			
UV intensity ^e , W/lamp	4,000			
Lamp Life, hours	5,000			
Fouling factor, %	90			
Lamp aging factor, %	89			
UV dose, mW-s/cm ²	40			
Physical Characteristics				
Channel dimensions, WxD	106" x 172"			
Number of channels	5 (4 plus 1 standby)			
Number of reactors per channel	1			
Number of banks per reactor	2			
Number of modules per bank	7			
Number of lamps per module	24			
Total number of lamps	1680			
Total power requirement, kW	5376			
Average power requirement, kW	2903			
Hydraulics				
Headloss, UV reactor only	9"			
Velocity in each channel, V, ft/s	1.74			
Liquid level control in channel	Motorized Weir Gate			

a Monthly permit limit 12 mg/L

The above design criteria are assumed based on available information and the current state of ultraviolet disinfection technology. A more extensive technology evaluation should be conducted prior to final design of the facility. Due to the extraordinary scale of this facility, CTE recommends the District undertake the following design process for selection and design of the UV disinfection equipment if final design is initiated:

 Request and evaluate independent, full-scale validation data (also known as biodosimetry data) from manufacturers of candidate disinfection systems for similarly sized units or the largest size for which the manufacturer has data available. This evaluation would provide an initial level-of-confidence that the candidate systems can achieve the target disinfection levels. Data should be from systems using the same bulb, ballast, and control technology as proposed for the full-scale system.

^b Annual average

^c Future requirement (monthly geometric average)

^d Mean value

^e 100% intensity at 100 hours of lamp use

- 2. Conduct a collimated beam testing program. This program would use site specific effluent and bacteria to determine the sensitivity of the site specific bacteria and pathogens to UV disinfection. The data would be used to size the UV lamps and reactors.
- 3. Increase frequency of UV transmittance testing at each plant to at least once per day for a period of one year or more to collect data on seasonal variability, daily variability, diurnal variability, and to capture the frequency of events that might reduce transmissivity such as wet weather and infrequent industrial discharges.
- 4. Conduct a more detailed life cycle cost analysis of the candidate disinfection systems based on the data collected during steps 1 through 3 above.
- 5. Construct a pilot testing facility (approximately 20 MGD, subject to change) designed to match lamp spacing, velocity profile and other design parameters of the proposed full scale units. The pilot testing facility would be used to determine:
 - a. Appropriate control sequences and optimization for the UV disinfection equipment, including appropriate sensing equipment to allow advanced power management.
 - b. In-situ disinfection performance including fouling rates of the lamps with and without ferric salt addition.
 - c. Design life of lamps and other UV system parts.
 - d. Actual M&O requirements in terms of labor and consumables as well as space requirements to complete required maintenance activities.
 - e. Performance of alternate equipment manufacturers, if alternates are available at the time of piloting.
 - f. Accuracy of life cycle cost analysis prior to final design of the full-scale system.
- 6. Conduct post-construction full-scale validation testing (biodosimetry testing) to confirm performance and determine operating parameters.

Using a program as described above, it may be possible to demonstrate the effective UV dosages to the regulators and optimize the equipment sizing criteria. For this study, reduction in the Illinois requirements for UV system sizing is not assumed based on the lack of data similar to that described above.

A budgetary cost (\$2,200,000) for a 20 mgd pilot facility has been included in the costs for implementation of the UV Disinfection Facilities. Costs for other portions of the design program are assumed to be

3.3 Low Lift Pump Station

Based on the analysis of hydraulics of the proposed improvements described in Section 2 above, it is estimated that the low lift pumps would be required to raise the water

approximately 7 feet (including static and friction losses) to the UV disinfection system influent, including estimated head to allow flow through the UV system. Should tertiary filtration become necessary in the future, these pumps would need to be modified to enable an increased head to approximately 11 feet (TDH) or more.

3.3.1 Basis of Design

Table 3.3-1 provides a summary of the basis of design for the Low Lift Pump Station.

Peak Flow, MGD 450 Average Flow, MGD 333 Minimum Flow, MGD 160 **Pumps** Type Axial Flow 6 total (N+1+1) Number Pumping Rates, gpm/pump 78,000 Total Dynamic Head, ft. 7 Motor, hp 250 Submergence, minimum, ft 14 Peak Power Demand, kW 515 Average Power Demand, kW 375 Wet Well Length, ft. 86 Width, ft. 101

Table 3.3-1 – Low Lift Pump Station Basis of Design

3.3.2 Pump Type

Several pump types were considered for this high flow (78,125 gpm) low head (7 feet TDH) application. Pump types considered included screw pumps, vertical turbine pumps, centrifugal pumps, and axial flow pumps. Many pump manufacturers found it difficult to recommend a pump that would operate efficiently for this application due primarily to the low head. Screw pumps and axial flow pumps appear to have the best operating performance for this condition.

Initially the Low Lift Pump Station would lift 450 MGD a total of 4 feet with a Total Dynamic Head (including station losses) of approximately 7 feet. However, if tertiary filtration is constructed in the future, the TDH would increase to approximately 11 feet (flow would remain the same). Screw pumps would not easily accommodate this change in head, without significant structural modifications to the pump station. However, axial pumps can be modified for future head conditions. Structural modifications to the pump station to accommodate these changes, if required, should be minimal. Therefore, axial flow, propeller type pumps are recommended. Vertically mounted units are readily available from manufacturers and were used for station layout. Horizontal units are also available, but are not as available as the vertical type.

3.3.3 Proposed Operational Description

The pump station would have a total of six pumps, with four duty pumps, one standby and one out of service (N+1+1). Four pumps would be driven by constant speed motors, two would be variable speed driven. In order to provide operational flexibility, the pump station would be divided into two wet wells, each containing three pumps. Normal wet well levels would be 14 to 16 feet Chicago City Datum (CCD). Design average flow (333)

MGD) can be handled by two constant speed and one variable speed pumps, leaving three pumps on standby. Peak flow (450 MGD) can be handled by four pumps, leaving two on standby. Minimum flow (160 MGD) would be handled by one constant speed pump and one variable speed pump at a low wet well level (~12-13 feet CCD) or two variable speed pumps at a normal wet well level. Typically, at least one variable speed pump would operate at all times, to handle fluctuations in flow. **Table 3.3-2** illustrates an example of pump operation at minimum, design average flow, and peak flow:

Flow, MGD	Pump Drive Type	Pump Flow, gpm	TDH, ft	Pump Eff.	Power Demand, kW
160 (5-year	Constant speed	74,000	7	84%	126
minimum)	Variable speed	39,000	2	80%	17
333 (Design	Constant speed	78,125	7	84%	126
Average)	Constant speed	78,125	7	84%	126
	Variable speed	75,000	6.5	83%	119
450 (Peak)	Constant speed	78,125	7	84%	126
	Constant speed	78,125	7	84%	126
	Constant speed	78,125	7	84%	126
	Variable speed	78,125	7	84%	133

Table 3.3-2 – Examples of Pump Operation

In order to eliminate vortices, pumps require a minimum submergence as a function of pump suction bell diameter. For this flow condition, a 96-inch suction bell is required, which requires a minimum submergence of 168 inches, or 14 feet. Submergence requirements should be verified by the pump manufacturer during final design.

Level sensors in the wet well would relay a signal to turn pumps on and off. Other control inputs that need to be monitored include discharge pipe pressure, flap gate position, and motor alarms.

3.3.4 Proposed Layout

Refer to Sheet C-102 for a proposed site layout of the LLPS and UV Disinfection Building. The space available for the construction of these facilities is constrained by the need to reserve space to the south for future tertiary filters and to the north by the CTA rail embankment. Flow would enter the pump station at the north end of the wet well, where it would be directed perpendicularly to the south through four 96-inch slide gates. Pumps are located at the south end of the pump station. An ideal pump intake approach per Hydraulic Institute standards was not possible due to the prohibitively long approach length required.

To accommodate the non-ideal pump intake approach, design features, which have been shown to be effective in other installations, were incorporated in this design in order to meet HI standards. For example, perforated plates, curtain walls, and floor and back wall splitters have been incorporated into the conceptual design. (See Volume 2 for a plan and section of the proposed layout). Sizing and details of these types of features are normally determined by physical scale modeling during detailed design. Furthermore, based on the total flow and flow per pump, the Hydraulic Institute recommends physical scale modeling.

4.0 NSWRP CIVIL

Due to constraints of the site related to the proposed location of the disinfection facilities, several significant civil improvements would be required. Those improvements include the following:

- 1. Temporary sheeting to support a railroad embankment
- 2. Construction of a permanent concrete retaining wall to allow locating the proposed facilities farther to the north
- 3. Construction of new roadways to access the new facilities and future tertiary filters
- 4. Construction of three gate structures and effluent conduits connecting the LLPS, UV Disinfection Building, and gate structures
- 5. Construction of associated utilities including stormwater collection, city water, effluent water, plant drain, electrical duct bank, and steam/condensate return.

In addition, available soil borings from previous projects indicate areas of concern related to the structure foundations. A discussion of the findings is also described below.

4.1 Basis of Design

Refer to the civil drawings in Volume 2 of this report for a layout of the proposed facilities on the site. The basis of design of each of the civil related improvements is presented below.

4.1.1 Temporary Sheeting

In order to support the existing rail embankment during construction of the retaining wall and connection to the existing effluent conduit upstream of Gate Structure #1, sheeting would be required to be installed along the embankment. The sheeting would be approximately 40-50 feet deep to support a cut into the embankment with a depth of up to 15 feet. It is assumed sheeting would be installed by vibratory pile drivers on weekends to minimize disruption to the CTA operating schedule. Additional shoring is assumed to be required to prevent movement of the sheeting and potential settlement of the rail. It is assumed that the sheeting would be abandoned in place.

4.1.2 Retaining Wall

In order to fit the proposed facilities onto the site, a cut would be required into the embankment. The embankment would be permanently supported by a 15 foot high concrete retaining wall. Soil anchors are likely to be required to provide additional support.

4.1.3 Roadways and Other Site Improvements

Proposed roadways associated with the UV Disinfection Facilities are intended to provide access to the structures and site for normal operations as well as allow access to heavy construction vehicles and delivery vehicles. The roadway would be constructed in accordance with District guidelines. It would be designed for AASHTO H-20 loading with an assumed reinforced portland cement concrete thickness of 12 inches. Curb and

gutter (standard 12 inch wide gutter with 6 inch curb) would be provided to facilitate maintenance and stormwater collection.

The existing site fence along the rail embankment and running south along the previous railroad right-of-way would be demolished to facilitate construction activities. A new 12' high, barbed wire, chain-link fence would be installed along McCormick Boulevard from the Pump and Blower House to the CTA rail abutment to enclose the new facilities.

The northern-most radio antenna would be relocated to the south to accommodate the new facilities and construction activities. It is assume that the cost for relocation would be borne by others.

4.1.4 Gate Structures/Effluent Conduits

Final effluent conduits connecting the various facilities associated with the UV Disinfection Facilities would be constructed along with the primary facilities. For the purposes of this study, the final effluent conduits are assumed to be square and 8' x 8' for the Battery E effluent to the LLPS and 11' x 11' for all other conduits. All effluent conduits would be cast-in-place concrete construction designed for open channel flow. Due to the comparatively low weight of the conduits and water contained therein compared to the soil excavated, no deep foundations are anticipated at this time. In most cases, the conduits would be designed for 2' or less cover and to handle H20 traffic loading with the exception of the conduit downstream of proposed Gate Structure #3, which is between 10 and 15 feet below grade to match the existing outfall conduit in that location. Where possible, common wall construction with adjacent structures is assumed to be utilized.

It should be noted that the LLPS Discharge Conduit would initially be designed for open channel flow, as are the remaining flow conduits. However, in the future, this conduit would be under pressure when the LLPS pumps are replaced to allow pumping to the tertiary filtration facility when it is constructed. As such, this conduit would be designed for pressure of approximately 15 feet of head above the top slab. In addition, two bonneted slide gates would be provided, in lieu of conventional slide gates, to account for this future change to the system operation. One would be on the conduit to the UV Disinfection Building and the other would be on the stub to the south for the future connection to the tertiary filtration facility.

Construction of the connection to the existing effluent conduit upstream of proposed Gate Structure #1 is assumed to require hand mining and extensive sheeting and shoring to allow exposure of the existing conduit. Underpinning of the existing and new conduits would be completed to prevent unexpected strain on the structure. A cast-in-place sleeve would be constructed around the existing conduit. The final connection would be made "in the wet" by removing the top of the existing concrete and inserting a pre constructed bulkhead along one side of the conduit. A water tight seal around the bulkhead is not likely to be possible and dewatering pumping is assumed necessary. It is assumed that plant flow would be controlled to maintain a narrow range of flows during this construction by diverting flows in excess of dry weather flow to TARP temporarily. The final connection would be made by sawcutting the opening and repairing the exposed surfaces before removal of the bulkhead and completion of the top of the connecting sleeve. Backfill would be structural fill and flowable fill if necessary. During this work, it is assumed that the CTA rail operation would be halted for a period of

two weeks and alternate means of public transportation (bus) would be provided for that period. That cost is not included in this study.

Three gate structures would be constructed to permit combining flows and facilitate flow control as follows:

Gate Structure #1

Gate Structure #1 is intended to combine flow from Battery A, B, C, D, and F on the existing site with flow from Battery E on the north site and direct it to the LLPS wet well. Motorized fabricated stainless steel slide gates (one on each flow source) would be provided to permit isolation of either flow source or to shut down the wet well. No aboveground structure would be associated with the gate structure, though its top would be 6 inches above grade. Guard rails and/or concrete bollards would prevent traffic over the gate structure to protect the motor actuators.

During the disinfection period (March to November) the gates would be normally open to permit flow to pass through the LLPS and UV Disinfection Building. During the winter period (November to April), the gates would be normally closed to allow bypass around the disinfection facilities. An access hatch would be provided to allow access to the structure.

Construction of Gate Structure #1 would be cast-in-place concrete. Due to the weight of the concrete and gates, pile foundations are assumed necessary. Where possible, common wall construction with adjacent structures would be utilized.

Gate Structure #2

Gate Structure #2 is intended to allow Battery E flow to bypass the LLPS and UV Disinfection Building. The structure is divided into two halves with a bypass connection between them; the north half directs Battery E flow from the east to Gate Structure #1 to the west and the south half directs UV Disinfection Building effluent to Gate Structure #3 to the east. Two motorized fabricated stainless steel slide gates, one on the downstream side of the Battery E conduit and one on the bypass connection, would be provided to allow bypass operation. No aboveground structure would be associated with the gate structure, though its top would be 6 inches above grade. Guard rails and/or concrete bollards would prevent traffic over the gate structure to protect the motor actuators.

During the disinfection period (March to November) the gate on the downstream side of the Battery E conduit would be normally open and the bypass connection gate would be normally closed. During the winter period (November to April), the gate positions would reverse to allow bypass around the disinfection facilities. Two access hatches, one on each half of the structure, would be provided to allow access to the structure.

Construction of Gate Structure #2 would be cast-in-place concrete. Due to the weight of the concrete and gates, pile foundations are assumed necessary.

Gate Structure #3

Gate Structure #3 connects the new UV Disinfection Building effluent conduit to the existing plant outfall conduit. This structure would also be used to convey flow through the UV Disinfection Facilities when required. A motorized fabricated stainless steel slide

gate on the upstream side of the existing outfall conduit would be provided. No aboveground structure would be associated with the gate structure, though its top would be 6 inches above grade. Guard rails and/or concrete bollards would prevent traffic over the gate structure to protect the motor actuator.

During the disinfection period (March to November) the gate would be normally closed to force flow from the existing site to be directed into the LLPS wet well through Gate Structure #1. During the winter period (November to April), the gate would be normally open to allow bypass of existing site flow around the disinfection facilities. An access hatch would be provided to allow access to the structure.

Construction of Gate Structure #3 would be cast-in-place concrete. Due to the weight of the concrete and gates and the soil conditions, pile foundations are assumed necessary. The base of the structure would form the connection to the existing plant outfall conduit. Underpinning of the existing and new conduits would be completed to prevent unexpected strain on the structures. The gate structure base would be constructed around the existing conduit. The final connection would be made "in the wet" by removing the top of the existing concrete and inserting a pre constructed bulkhead along one side of the conduit. A water tight seal around the bulkhead is not likely to be possible and dewatering pumping is assumed necessary. It is assumed that plant flow would be controlled to maintain a narrow range of flows during this construction by diverting flows in excess of dry weather flow to TARP temporarily. The final connection would be made by sawcutting the opening and repairing the exposed surfaces before removal of the bulkhead.

Following the final completion of the connection, a second and third full pipe diameter bulkhead would be constructed upstream and downstream of the proposed gate in the existing plant outfall conduit to allow its installation. Plant flow would be diverted through the UV Disinfection Facilities during this work. Underwater construction techniques would be required to make the insertion and sealing of the bulkheads. Following installation of the gate, the bulkheads would be removed and the gate structure would be completed to grade.

Costs for the gate structures and special connections have been included in the opinion of probable construction cost included in Appendix F.

4.1.5 Site Utilities

Site utilities would be demolished, rerouted, and constructed to support the new facilities. The following utilities would be demolished or rerouted as shown on Sheet C-103:

- 1. Wash Water Supply Rerouted
- 2. Non-Potable Water Supply Rerouted
- 3. Plant Effluent Rerouted
- 4. Site Sprinkler Demolished and Capped
- 5. Miscellaneous Site Drainage Demolished

The following site utilities would be added to support various functions within the new UV Disinfection Facilities:

- 1. Steam and Condensate Return Piping Constructed from Battery A service tunnel to head of LLPS and UV Disinfection Buildings
- 2. City Potable Water Routed from current location near Process Water Building to LLPS and UV Disinfection Building for potable water use.
- 3. Non-Potable Water Routed from existing piping (rerouted to accommodate new facilities) to LLPS and UV Disinfection Building for non-potable water use (wash down).
- 4. Plant Drain New plant drain installed along south side of UV Disinfection Facilities to existing 6'-6" interceptor along eastern side of existing site and connected to LLPS and UV Disinfection System.
- 5. Stormwater Collection Installed to collect stormwater runoff from new buildings and roadway and routed to plant drain.
- 6. Electrical Duct Bank Routed from new power substation on Battery E site to UV Disinfection Building for power distribution.

4.1.6 Geotechnical Information

The project team has reviewed the two sets of boring logs (1969 and 1977) for the proposed site of UV Disinfection Facilities. These logs were used to understand the general subsurface soil conditions at the proposed site and provide a preliminary opinion on suitable foundation type for the proposed facilities. See Appendix E for copies of the referenced boring logs.

The quality of the boring logs is poor, and the properties of soils such as unit weights and consolidation are not reported. Boring logs show that soil conditions are highly variable across the proposed site (e.g., thickness of soft silty clay layer is 19 feet at B-1 and 40 feet at B-2; both B-1 and B-2 are close to each other). In general, fill and topsoil are encountered near the ground surface. A stiff silty clay layer underlies the fill/topsoil layer. Below the stiff silty clay layer, a very soft silty clay layer (with very low unconfined compressive strength and high moisture content) is encountered to depths ranging from 30 to 53 feet below the ground surface. Inter-bedded silt and sand layers and hardpan are encountered before finally experiencing auger refusal (apparent top of the bedrock) at depths ranging from 55 to 58 feet.

The proposed structures would be located 15 to 20 feet below the existing grade. This means that the base of the structures would be located within the soft silty clay layer. Approximately 25 to 35 feet of soft clay would remain below the base of the proposed structures. If a slab foundation is proposed, a detailed analysis for bearing capacity and settlement is warranted. Since the thickness of soft clay and underlying soils are widely varying, a detailed assessment of differential settlement is also necessary. Such analysis is not possible now based on very limited information presented on the boring logs.

As the proposed structures are large and heavy, and also the maximum allowable settlement should be less than one inch, it is appropriate at this level of design development to assume a deep foundation system extending into the hardpan or to the top of the bedrock. Either drilled shafts or pile foundations are suitable to use. However,

there may be potential squeeze-in problems in the soft clay layer during the construction of drilled shafts, even though some preventive measures (e.g. use of casing) may be taken to avoid such problems. In our opinion, a pile foundation system should be considered for cost estimate purposes.

It is assumed for this analysis that the effluent flow conduits would be significantly lighter than the other structures and approximate the weight of the soil to be removed.

A detailed subsurface investigation is recommended to characterize the soft silty clay layer and underlying soil layers. Both strength and consolidation properties of these soils should be determined by field and laboratory testing. These data would be necessary for the final selection and design of the foundation system.

5.0 NSWRP STRUCTURAL AND ARCHITECTURAL

5.1 Introduction

The objective of this Section is to document the design criteria for the structural, architectural components of this project, including recommendations, allowable stresses, and loadings that would be used in designing the new project structures and modifying existing structures. Refer to the structural and architectural drawings in Volume 2 of this report.

5.1.1 Codes and Specifications

The following codes would be used in addition to the general design standards listed in Section 1.2:

- The International Building Code 2003 (IBC) Village of Skokie
- The International Fire Code 2003 (IFC)
- NPFA 101, Life Safety Code, 1997 Edition
- OSHA, United States Department of Labor, Occupational Safety and Health Administration, Latest Edition
- Building Code Requirements for Structural Concrete, (ACI 318-02) and Commentary, (ACI 318R-02).
- Code Requirements for Environmental Engineering Concrete Structures, ACI 350-01) and Commentary (ACI 350R-01).
- Seismic Design of Liquid Containing Concrete Structures, (ACI 350.3-01), and Commentary, (ACI 350.3R-01).
- ACI "Manual of Concrete Practice", 2005, American Concrete Institute, Detroit, MI.
- ACI Committee 315, "Details and Detailing of Concrete Reinforcement, ACI 315-99.
- Specification for Structural Steel Buildings Allowable Stress Design and Plastic Design, Ninth Edition, June 1, 1989
- Manual of Steel Construction Allowable Stress Design, Ninth Edition, 1989
- Building Code Requirements for Masonry Structures and Commentary, ACI 530-02, ASCE 5-02/TMS 402-02 and Specification for Masonry Structures and Commentary, ACI 530.1-02/ASCE 6-02/TMS602-02.
- American Society of Civil Engineers, Minimum Design Loads for Buildings and Other Structures, ASCE 7-02.

- American Association of State Highway and Transportation Officials, AASHTO, Standard Specifications for Highway Bridges, Seventeenth Edition, 2002
- Soil Boring Logs in Contract 78-020-CP For Secondary Treatment Facilities at the North Side Sewage Treatment Works.
- The Illinois Accessibility Code 2004.
- The Metropolitan Water Reclamation District of Greater Chicago (MWRDGC) "Standard Specifications".
- The MWRDGC Design and Construction Manual, "Engineering Standards".
- United States Naval Facilities Command (NAVFAC), September 1986, "Design Manual 7.02, Foundations and Earth Structures".
- CFR 29 Parts 1900-1910.999 and Part 1926, OSHA
- American Society for Testing Materials (ASTM) Standards.
- American Welding Society, ANSI/AWS D1.1-98, "Structural Welding Code Steel"

5.1.2 Loads

The following design loads would be used for the proposed structures:

Tanks, Channels and Structures Below Grade:

- Hydrostatic liquid pressure-operating water level/flood water level 62.4 psf.
- Lateral earth pressure for active, at-rest and passive conditions Per Geotechnical Report (lateral load due to surcharge loading of H-20 truck would be added).
- Surcharge Load 3 feet of soil.
- Frost depth Minimum 3'-6" below finished grade.
- Design high ground water table elevations. All new structures would be checked for buoyancy for the case of high ground water table at finished grade and dead load of the structure only and is described in Part 6.1.4 below.

Roof Slab at or below Grade:

- DL: Weight of concrete slabs
- SDL: Backfill and other superimposed dead loads including underhung ancillary equipment and piping
- LL: The equivalent of 3 feet of soil or H-20 truck loading whichever governs

Buildings and Miscellaneous Structures:

• Loadings for design of the building would be obtained from appropriate codes; however, certain minimum loads would be used as shown in Part 6.1.2.3 below.

Minimum Uniform Live Loads:

- Checkered Plate 150 psf
- Grating 100 psf
- Stairs and catwalks 100 psf
- Electrical control rooms: 250 psf Estimate support area and equipment weights and assume loads

- applied anywhere in area
- Heavy Equipment rooms 300 psf
- Dismantling and storage
- Storage areas: 150 psf Determine reasonable stacking height and type of stored material
- Shop floors: 150 psf
- Garage floors: 150 psi
- Truck wheel loads per AASHTO and as appropriate
- All other: 150 psf
- Fastest mile wind speed (miles per hour): 75 mph
- Snow (minimum): 30 psf Snow drift loads would be checked where applicable in addition to all top supported and under hung ancillary equipment and piping
- Underhung piping and equipment in addition to the required: 50 psf minimum live load
- Equipment live load plus 50 psf on adjacent areas, or minimum uniform live load, whichever is greater

Seismic Requirements – Cook County:

Buildings and Non-Liquid Containing Structures (IBC):

- Seismic use group Group II
- Seismic design category
- Seismic Importance Factor 1.25
- Spectral response acceleration for short period (SDS) 0.192
- Spectral response acceleration for 1 second period (SD1) 0.10
 - o Soil profile name Stiff soil profile
 - Site class

Liquid Containing Structures (ACI 350.3-01):

• Seismic zone factor 0

5.1.3 Design Stresses

The following stresses would be used for design of the structures:

Concrete and Reinforcing Steel:

Liquid Containing Structures:

- Use ACI 350-01, Code Requirements for Environmental Engineering Structures (ACI 350-01) and Commentary (ACI 350R-01) and Seismic Design of Liquid Containing Concrete Structures (ACI 350.3-01) and Commentary (ACI 350.3R-01).
- Concrete compressive strength at 28 days : fc' = 4,500 psi
- Reinforcing steel (A 615, Gr. 60) flexural stress: fy = 60,000 psi

Building and Non-Liquid Containing Structures:

- Use Strength Design Method of Building Code Requirements for Structural Concrete (ACI 318-02) and Commentary (ACI 318R-02).
- Concrete compressive strength at 28 days: fc' = 4,500 psi
- Reinforcing steel (A 615, Gr. 60) flexural stress: fy = 60,000 psi

Structural Steel

- Conform to the AISC Specification for Structural Steel Buildings Allowable Stress Design and Plastic Design, Ninth Edition, 1989, and the Manual of Steel Construction, Allowable Stress Design utilizing the following materials.
- ASTM A 992 for W shapes, unless otherwise specified
- ASTM A 36 for angles plates and bars
- ASTM A 325 high strength bolts
- ASTM A 307 or A 36 bar stock for anchor bolts

5.1.3 General Design

The following reinforced concrete structures would contain continuous PVC waterstops at all vertical and horizontal construction and expansion joints in walls and slabs:

- 1. All fluid containing structures.
- 2. All basements and below ground structures with one surface in contact with soil or water and the opposite surface dry and exposed.

Fluid applied waterproofing would be applied to the exterior surfaces of all walls with one surface in contact with soil and the opposite surface dry and exposed.

All structures below grade, including, but not limited to, basements, tanks, and other buried structures, would be designed to resist buoyancy for a groundwater table at finished grade. Only the dead weight of the concrete structure below ground and soil on the foundation footings around the outside of buildings, tanks, and other buried structures would be relied on to resist buoyancy. Pressure relief valves and/or perimeter drains and sump pits with pumps would not be used to resist buoyancy.

All access hatches would be stainless steel. Handrails would be stainless steel.

5.1.4 Foundation Design

The foundation design for the various structures was based on existing available borings and interpretations of these borings by an independent Geotechnical Engineer for use in estimating foundation costs for this preliminary phase of work. Based on this information, it was determined that a pile foundation is required to support the UV Facility and the Low Lift Pump Station, and for the purposes of this study, it would be

assumed that 40 ton capacity, concrete filled pipe piles would be required for the support of the UV Facility and the Low Lift Pump Station.

Prior to final design, a detailed subsurface investigation should be undertaken to characterize the soils, including soil borings, interpretation of the borings and for the final selection of the type of foundation that would be required.

5.2 NSWRP UV FACILITY

The new UV Facility would be a one story reinforced concrete building with five (5) channels for the five (5) UV Reactors, an electrical room, a storage room, a control room and an effluent sampling room. The exterior wall construction would be a non-load bearing composite cavity wall composed of concrete masonry units, airspace, insulation and an exterior face brick. The exterior masonry materials and detailing would be similar to existing onsite masonry structures.

The roof structure would be constructed using one-way, cast-in-place reinforced concrete slabs spanning cast-in-place reinforced concrete beams. The beams would be supported by cast-in-place reinforced concrete columns. The roofing would be composed of fully adhered cold applied roofing membrane over tapered rigid insulation. The roof drainage would be directed to scupper boxes at the perimeter of the building. The scupper boxes would connect to downspouts leading drainage to grade. Aluminum skylights would be provided over each reactor to permit natural light into work areas. An aluminum framed window would be provided in the control room for visual access to the UV reactor room.

Personnel doors would be stainless steel frames and doors. The double doors in the electrical room would have a removable transom to provide access for large equipment. The overhead door would be an insulated aluminum coiling door. Specialty floor hatches would be provided to accommodate the UV equipment maintenance. The interior floor finish in the building would be hardened concrete outside of the control room and effluent sampling room. The control room and effluent sampling room would have suspended acoustic ceilings and resilient tile flooring. Interior partitions and concrete structure would be painted.

The entire substructures, including channels and foundation grade beam/walls would be constructed of cast-in-place reinforced concrete supported on concrete filled pipe piles. Gratings in the UV Reactor Room would be galvanized steel with galvanized steel perimeter angles and supports.

5.3 LOW LIFT PUMP STATION

The new LLPS would be a 40'+ steel supported building (one story) with a pump room and an electrical room. The exterior wall construction would be a non-load bearing composite cavity wall composed of concrete masonry units, airspace, insulation and an exterior face brick. The exterior masonry materials and detailing would be similar to existing onsite masonry structures.

The roof structure would be constructed using standard galvanized roof decking to span the steel support beams. The beams would be supported by steel columns. The roofing would be composed of fully adhered cold applied roofing membrane over tapered rigid insulation. The roof drainage would be directed to scupper boxes at the perimeter of the

building. The scupper boxes would connect to downspouts leading drainage to grade. Removable, double hip-type, aluminum, structural skylights would be provided over each pump to permit natural light into work areas and removal of the pumps by crane in the future.

Personnel doors would be stainless steel frames and doors. The double doors in the electrical room would have a removable transom to provide access for large equipment. The overhead door would be an insulated aluminum coiling door. The interior floor finish in the building would be hardened concrete. Interior walls and concrete structure would be painted.

The entire substructures, including channels and foundation grade beam/walls, would be constructed of cast-in-place reinforced concrete supported on concrete filled pipe piles.

6.0 NSWRP ELECTRICAL

6.1 Codes/Standards

The following codes and standards are required for this project.

- NFPA-70 National Electrical Code, 2002 or latest adopted by the Village of Skokie.
- NFPA-820 Fire Protection in Wastewater Treatment and Collection Facilities, 2003.
- Institute of Electrical and Electronics Engineers (IEEE).
- MWRDGC GS, February 1997.
- MWRDGC GSE, March 1994.
- Underwriters Laboratories (UL).
- National Electrical Manufacturer's Association (NEMA).
- Insulated Power Cable Engineers (IPCEA).
- Illuminating Engineering Society (IES).

6.2 Electric Service

A redundant electric service to the UV Disinfection Facility and the Low Lift Pump Station would be provided. A new electric service transformer yard is planned for the new Battery E to the north of the Chicago Transit Authority right of way. Facilities would be provided at the Battery E service from ComEd for service to UV Disinfection Facility.

A medium voltage cable in underground ductbank would be provided from the Battery E service location to supply the UV Disinfection Facility.

In addition, per ComEd policy, the District would be responsible for costs to upgrade ComEd transmission system improvements required to provide the new electrical power to the new transformer yard near Battery E. ComEd improvements would include:

- 1. Protective device adjustment at TSS-85 (substation immediately north of existing WRP site).
- 2. Underground conduit and cable to provide service to the new District transformer yard adjacent to the proposed location for Battery E.

- 3. New overhead power transmission line from intersection of Skokie Boulevard and Oakton Street to provide redundant power service from separate substation (TSS-88 located in Skokie, Illinois at Church Street and I-94).
- 4. Protective device adjustment at TSS-88.

The proportional costs for UV disinfection for the ComEd improvements and the new electrical service transformer yard are included in the overall estimate for disinfection as shown in Section 10. The proportional cost would be 70% of the total cost based on the UV power demand of 5 MVA versus the total power demand of 7 MVA (5 MVA for UV disinfection + 1 MVA for Battery E + 1 MVA for future improvements).

6.2 System Grounding

Electrical systems shall be solidly grounded. Grounding shall be in accordance with the National Electrical Code and the Chicago Electrical Code for equipment grounding and bonding conductors for grounding raceway and equipment.

6.3 Conduit

Exposed conduit shall be PVC coated Rigid Galvanized Steel Conduit. Conduits in non-finished areas shall be installed either exposed on the surface of the structure or concealed in concrete floor slabs or below grade. Conduits below grade outside of the building shall be rigid steel and shall be encased in reinforced concrete. Ductbanks shall have 50 percent spare conduits.

Conduits shall conform to MWRDGC General Specifications: Electrical (GSE) Table 1 (Page GSE-8).

Spacing of supports for exposed conduit shall conform to MWRDGC GSE Table 3 (Page GSE-10).

6.4 Wire

600 volt Insulated copper conductors in conduit shall be provided for all power, control, alarm, instrumentation, signal, lighting and grounding installations, unless otherwise indicated. The insulation shall meet ANSI/NFPA 70. The wire and cable shall conform to the MWRDGC GSE Table 4 (Page GSE-10).

Medium voltage cable shall be ethylene propylene rubber (EPR) insulated cable, U.L. listed and labeled MV-90, 133% insulation level, single conductor copper, Class B strand.

6.5 Motors

Motors 1/2 horsepower and larger shall operate on 480 volt, 3-phase, AC power supplies, and motors smaller than 1/2 horsepower shall operate on 120 volts, single phase, AC power supplies.

6.6 Emergency Systems

The emergency system for new areas would be supplied from the existing emergency supply. Emergency lights would have unit batteries to provided final reserve source of current supply.

Emergency lighting and exit signage would be provided as per code requirements to illuminate the path of ingress/egress in emergency situations. Separate emergency lighting panels (EP) would be provided as per the Village of Skokie electrical code.

6.7 Lightning Protection

New structures shall be protected by a lightning protection system. The system shall be a conductor system protecting the entire building and consisting of copper air terminals on the building roof parapets; grounding electrodes; and copper interconnecting conductors.

The system shall be designed in accordance with ANSI/NFPA 780 - Lightning Protection Code and shall have a UL Master Label. The lightning protection system components shall conform to ANSI/UL 96 - Lightning Protection Components.

6.8 Specific Electrical Equipment

The basis of specific design equipment is described below.

6.8.1 Medium Voltage Switchgear

Table 6.8.1-1 describes medium voltage switchgear. **Table 6.8.1-2** describes the criteria to be used for circuit breakers.

Table 6.8.1-1
Medium Voltage Switchgear Criteria

Medium voltage Switchgear Criteria			
Item	Criteria		
Туре	Medium Voltage Metal-clad Draw-out Switchgear		
Otandanda			
Standards	■ NEMA SG.5		
	• ANSI C37.20.2		
Rated Voltage	13,200 Volts		
Number of phases	3		
Bus Material	Tin plated copper		
Rated BIL	95,000 Volts, to be coordinated with surge		
	arrester rating		
Minimum Main Bus Rated Ampacity	2,000 Amperes		
Minimum interrupting capacity	500 MVA		
Manufacturer	Cutler Hammer.		
	- ABB - ASEA Brown Boveri.		
	 Siemens Energy and Automation. 		
	Approved equal.		
Metering Type	Solid State Multifunction		
Metering Location	Main circuit breaker and other critical feeder		
	circuit breakers		
Relaying Type	Solid state multifunction		
Relaying Manufacturer	Schweitzer Engineering Laboratories, SEL		
	Areva NP Co.		
	Approved equal		
Enclosure Rating	NEMA 1		

Table 6.8.1-2
Circuit Breaker Ratings and Features Criteria

Item	Criteria	
Туре	Draw-out carriage type with racking mechanism.	
	 Circuit breakers shall be vacuum type. 	
Operator Voltage	Electric, 125 Vac	
Controls	Manually operated electric controls with piston grip switches and indicator lights. Location would be coordinated with Arc Flash analysis.	
Minimum circuit breaker frame current rating.	1,200 Amperes	
Manufacturer	Same as Switchgear manufacturer	

6.8.2 Pad Mounted Transformers

Table 6.8.2-1 provides the design criteria for pad-mounted transformers.

Table 6.8.2-1
Pad-Mounted Transformer Criteria

rau-woulden Fransionner Chteria		
Item	Criteria	
Type	Outdoor, Oil-filled Power Transformer	
Primary connection type	Elbow Type terminators	
Primary Voltage	13,200 Volts	
Primary Number of phases	3	
Primary wiring configuration	Delta connection, 3-wire	
Secondary Connection type	Bolt-on type bushing	
Secondary Voltage	480/277 Volts	
Secondary Number of phases	3	
Secondary wiring configuration	4-wire, grounded	
Efficiency	Peak efficiency point of pad mounted transformers to be at 50% of efficiency rating.	
Capacity	2,000 kVA or as required	
Primary BIL	95,000 Volts, to be coordinated with surge protection rating	
Secondary BIL	30,000 Volts, to be coordinated with surge protection rating	
Nominal Impedance	5.75 percent	
Temperature Rise	55/65 Degrees C	
Transformer insulating oil	Non-flammable, environmentally safe insulating fluid	
Manufacturers	ABB - ASEA Brown Boveri	
	Cooper Power Systems (RTE)	
	Square D	
	General Electric	
	 Approved equal 	

6.8.3 Secondary Unit Substation

Table 6.8.3-1 summarizes the design criteria for secondary unit substation. **Table 6.8.3-2** provides the criteria to be used for the low voltage distribution.

Table 6.2.3-1 Secondary Unit Substation

Secondary offic Substation		
Item	Criteria	
Туре	Radial Secondary Unit Substation with	
	close coupled air terminal compartment and close coupled Secondary Low	
	and close coupled Secondary Low Voltage Switchgear	
Standards	NEMA 210	
Transformer Type	Dry type	
Transformer insulation system	Vacuum pressure impregnation with	
Transferrior mediation system	polyester resin (VPI)	
Primary equipment	Air terminal compartment	
Primary Voltage	13,200 Volts	
Primary Number of phases	3	
Primary wiring configuration	Delta connection, 3-wire	
Secondary Connection type	Bolt-on type bushing	
Secondary Voltage	480/277 Volts	
Secondary Number of phases	3	
Secondary wiring configuration	4-wire, grounded	
Efficiency	Peak efficiency point of transformers to	
	be at 50% of efficiency rating.	
Capacity	1,500-2,000 kVA or as required	
Primary BIL	95,000 Volts, to be coordinated with	
	surge protection rating	
Secondary BIL	10,000 Volts, to be coordinated with	
Winding Motorial	surge protection rating	
Winding Material	Copper	
Nominal Impedance	5.75 percent	
Temperature Rise	80 Degrees C	
Minimum K factor	K4	
Accessibility	Front and rear	
Enclosure Rating	NEMA 1	
Manufacturers	Cutler-Hammer.	
	- ABB - ASEA Brown Boveri	
	• Square D	
	General Electric	
	 Approved equal 	

6.8.4 Low Voltage Switchgear

Table 6.8.4-1 provides the design criteria for low voltage switchgear. **Table 6.8 .4-2** lists the criteria for circuit breakers.

Table 6.8.4-1 Low Voltage Switchgear Criteria

Item	Criteria
Type	Type DS, Metal-Enclosed Drawout Switchgear
Enclosure Rating	NEMA 1
Standards	NEMA SG-5
	ANSI C37.20.1
	UL 1558
Rated Voltage	480 Volts
Number of phases	3
Bus Material	Tin plated copper
Minimum Main Bus Rated Ampacity	3000 Amperes
Minimum interrupting capacity	100 kA
Accessibility	Front and rear
Manufacturer	Cutler Hammer
Metering Type	Digital Solid State multifunction meters
Metering Location	Main circuit breaker and other critical feeder
	circuit breakers
Relaying Type	Solid state multifunction
Relaying Manufacturer	Schweitzer Engineering Laboratories, SEL
	Areva NP
	Approved equal

Table 6.8.4-2 Circuit Breaker Ratings and Features Criteria

Item	Specifications		
Туре	Draw-out carriage type with racking mechanism.		
Standards	NEMA SG-3 ANSI C37.13, C37.16, C37.17 UL 1066		
Operators	Manual		
Controls	Manual with position indicator lights.		
Minimum circuit breaker frame current rating.	800 Amperes		
Manufacturer	Same as Switchgear manufacturer		

6.8.5 Motor Control Centers

The design criteria for motor control centers are summarized in Table 6.8.5-1.

Table 6.8.5-1
Motor Control Center Criteria

Item	Criteria
Rated Voltage	480 Volts
Number of phases	3
Main bus minimum current rating	600 Amperes
Bus Material	Tin-plated Copper
Minimum short circuit rating	100,000 Amperes
Accessibility	Front only
Wiring class	NEMA Class II-S, Type B.
Overload Protection type	Solid State Type.
Metering type	Digital Solid State multifunction meters.
Enclosure type	NEMA 12
Manufacturer	Cutler-Hammer.
	Allen Bradley.
	Square D Corp.
	 Siemens Energy and Automation.
	Approved equal

7.0 NSWRP INSTRUMENTATION SYSTEM

The control of the process equipment shall be integrated into the existing DCS System which is provided by ABB.

The monitoring and control of the Low Lift Pump Station and the UV Disinfection Facility would be provided via the plant DCS System. Manual local control of the equipment would be provided. See Section 4.0 for a description of the control philosophy for the LLPS pumps and the UV Disinfection System.

7.1 Applicable Codes and Standards

Where applicable, the latest version of the codes and standards from the following institutions/organizations would govern the design:

- National Electrical Code (NFPA 70) with Village of Skokie local amendments.
- National Fire Protection Association (NFPA) standards:
- NFPA 820 Fire Protection in Wastewater Treatment and Collection Facilities
- Underwriter's Laboratories (UL)
- Illuminating Engineering Society of North America (IESNA)
- Institute of Electrical and Electronic Engineers (IEEE)
- National Electrical Manufacturers Association (NEMA)
- National Electrical Contractors Association (NECA)
- MWRDGC Standard Details and Specifications
- Variable Frequency Drives Reference Standards
- American National Standards Institute (ANSI)
- ANSI/IEEE 519 IEEE Guide for Harmonic Control and Reactive Compensation of Static Power Converters.

- ANSI/IEEE 597 IEEE Practices and Requirements for General Purpose Thyristor DC Drives.
- National Electrical Manufacturers Association (NEMA)
- NEMA ICS 3.1 Safety Standards for Construction and Guide for Selection, Installation and Operation of Adjustable-Speed Drive Systems.
- NEMA ICS 7 Industrial Control and Systems: Adjustable Speed Drives.

8.0 NSWRP MECHANICAL AND PLUMBING

8.1. Mechanical Codes

Where applicable, the latest version of the codes and standards from the following institutions/organizations would govern the design:

- The International Mechanical Code 2003
- The International Plumbing Code 2003
- National Fire Protection Codes (NFPA), Section 820, 2007
- American National Standards Institute (ANSI)
- American Society For Testing Materials (ASTM)
- American Society of Heating, Refrigerating and Air-Conditioning Engineers (ASHRAE)
- SMACNA HVAC Duct Construction Standards
- International Building Code 2003

8.2 Basis of Design

The UV Disinfection Building and the LLPS would follow the International Building Codes for fire protection pending future direction by the District.

8.2.1 Ventilation Rates

The ventilation rates are selected based upon the need to conform to the recognized national standards applying to wastewater treatment plants. Specifically, NFPA 820, "Standard for Fire Protection in Waste Water Treatment and Collection Facilities" and the "International Fire Code" are used for the design.

8.2.2 Design Temperatures

Design temperatures are based upon local climatic data found in the latest edition of ASHRAE Handbook of Fundamentals

8.2.2.1 Heating

The design space temperature for all process areas would be 55°F with an outdoor air temperature of -10°F. The design space temperature for occupied areas would be 70°F.

8.2.2.2 Air Conditioning

The design space temperature and humidity conditions for areas requiring air conditioning would be 78°F DB, 50% RH with an outdoor air condition of 91°F DB, 75°F WB. Summer ventilation only spaces would have a maximum design space temperature rise of 15°F.

8.2.3 Plumbing

The plumbing systems for the UV Disinfection Building and LLPS would be designed to the "International Plumbing Code", 2003.

8.2.3.1 Potable Water

Potable water would be supplied to the wash sink in the UV Disinfection Building from plant potable water distribution system.

8.2.3.2 Effluent Water (aka Plant Service Water)

Effluent water would be available from the plant effluent water distribution system. Effluent water would be provided for equipment wash down in the UV Disinfection Building and the LLPS.

8.2.3.3 Sanitary Drainage

General floor drainage would be provided in all rooms as required by codes. Drainage from the wash sink and the effluent water sampling sink would be routed to the plant sanitary drain. Floor traps and sink traps would be vented.

8.2.3.4 Fire Protection

The fire protection system would consist of portable fire extinguishers and fire hydrants, in accordance with the requirements of NFPA 820 and local code requirements.

8.3 Proposed Mechanical and Plumbing System

The following section details the proposed equipment and operation.

8.3.1 UV Disinfection Facility

Air-conditioning would be provided for the operator control room. Heating for the electrical room would be provided by electric unit heaters. Heating for process and storage rooms would be provided by steam unit and space heaters.

Summer ventilation for the electrical room and filter room would be designed for a maximum space temperature increase of +15°F over ambient. Temperature control would consist of cycling exhaust fans that are interlocked with outside air intake dampers. Summer ventilation for the effluent sampling room and storage room would consist of 4 air changes per hour. Exhaust fans for the electrical room would consists of two exhaust fans at 50% design capacity and one standby exhaust fan. Exhaust fans for the UV disinfection room would be sized for 2/3 design capacity.

Effluent hydrants and hose reels would be provided for wash down of the UV system at the north and west doors. Potable water would be provided to the wash sink at the west door. An inline instant water heater would be provided for domestic hot water.

8.3.2 Low Lift Pump Station

Heating for the electrical room would be provided by electric unit heaters. Heating for the pump room would be provided by steam unit heaters.

Summer ventilation rates for the electrical room and the pump room would be designed for a maximum space temperature increase of +15°F over ambient. Temperature control would consist of cycling exhaust fans that are interlocked with outside air intake

dampers. Exhaust fans for the electrical and pump room would consists of two exhaust fans at 50% design capacity and one standby exhaust fan.

9.0 NSWRP AREAS REQUIRING FURTHER ANALYSIS

The following areas require further analysis as part of a preliminary design effort prior to final design of the proposed facilities.

- A detailed subsurface investigation is recommended to characterize the soft silty clay layer and underlying soil layers. Both strength and consolidation properties of these soils should be determined by field and laboratory testing. This data would be necessary for the final selection and design of the foundation system by a qualified geotechnical engineer.
- More complete investigation of rail embankment and methods required for construction of the retaining wall and connection to the existing plant effluent conduit is recommended. Specifically, depending on the method of construction and fill material of the embankment, dewatering efforts could be substantially greater than those assumed for this report.
- 3. A more detailed evaluation of the layout and location of the gate structures and flow conduits is recommended. A more optimal arrangement that may consolidate several functions could be developed during preliminary design.
- 4. A more detailed evaluation of potential pump types and arrangements for the LLPS. Historically, horizontal arrangements, similar to the existing Wilmette Lock pumps, have been used in flood control projects that might be applicable here.
- 5. A more detailed evaluation of large-scale M&O requirements for the selected UV technology is recommended to ensure the appropriate equipment spacing, operations rooms, and storage space is provided in the new facilities. Existing large-scale facilities are either based on older technology or are operated intermittently as wet weather facilities. A pilot facility is recommended to provide this information. Estimated construction cost is \$2,200,000 not including operational costs or performance evaluation.
- 6. Physical scale modeling during preliminary design of the LLPS is strongly recommended per Hydraulic Institute Standards for a pump station of this size and given the deviation from the ideal inlet configuration.
- 7. Costs for addition of a ComEd Substation are not currently included in the costs for implementation of the UV Disinfection Facilities, although a new substation on the north site would be required.

10.0 NSWRP PRELIMINARY COST OPINION

A preliminary opinion of probable construction (OPCC) of the North Side WRP UV Disinfection Facilities is estimated at approximately \$108.8 million including engineering and administrative costs as shown in **Table 10.0-1**, which also presents annual operating costs and a 20-year net present worth value for the project. Annual operating costs are based on the facilities operating from March to November each year. Appendix F provides detailed line item summary tables for capital and M&O estimates.

The estimated construction cost is based on June 2007 dollars represented by an Engineering News Record (ENR) Construction Cost Index (CCI) of 7983.

Table 10.0-1 – NSWRP UV Disinfection Facilities Preliminary OPCC and M&O Costs

Capital Cost Estimates		
NSWRP UV Pilot Plant	\$2,200,000	
ComEd Service Upgrade Charge	\$2,900,000	
A. General Sitework	\$27,200,000	
B. Low Lift Pump Station	\$27,000,000	
C. Disinfection System	\$49,500,000	
Total Capital Cost	\$108,800,000	
Maintenance & Operations Cos	st Estimates	
A. General Sitework	\$130,000/yr	
B. Low Lift Pump Station	\$1,100,000/yr	
C. Disinfection System	\$3,590,000/yr	
Total Annual M&O Cost	\$4,830,000/yr	
Total Present Worth M&O Cost	\$111,900,000	
Total Present Worth	\$220,700,000	

All costs in 2007 dollars.

Per District guidelines, this opinion is categorized as a Level 3 as defined by the Association for the Advancement of Cost Engineering Recommended Practice No. 18R-97 and represents a conceptual estimate with an expected deviation range from actual cost of -15% to +30% assuming no substantial change in scope or extraordinary events and not including escalation from the date of this report to the start of construction.

10.1 **Basis of Opinion of Capital Cost**

The assumptions made used to develop the capital costs for the proposed facilities are summarized below and/or described in the previous sections:

- Design Flow: Maximum design flow was used (NSWRP = 450 mgd).
- Proposed Effective Disinfection Limit (E. Coli, cfu/100 ml): 400 monthly geomean for North Side.
- UV Disinfection:
 - UV Transmission: 65% minimum per IEPA standard
 - UV Dosage: 40 mJ/cm² per UV_{dis} sizing software
- Each plant would disinfect effluent from March 1 through November 15. During the remaining months, the disinfection facilities, including LLPS, would be bypassed.
- Cost opinions were divided into the following categories:
 - Site Work
 - Low Lift Pump Station
 - UV Disinfection Building
- Costs for major equipment were obtained from the following vendors:

<u>Vendor</u>
Trojan Technologies, Inc.
Sulzer Pump, Morrison Pump
Rodney Hunt
Rodney Hunt, Whipps

UV channels were enclosed in a UV building.

Redundancy

- UV multiple channels were used to meet the effluent limit at peak flow with one channel out of service.
- Pumps were provided with N+1+1 redundancy per the District's standard guidelines.

10.2 Basis of Operation and Maintenance Costs

The assumptions used to develop the maintenance and operating costs are presented below:

- A power cost of \$0.0684/kW-hr was used as a composite rate based on the District's 2007 power supply contract.
- Labor rates were developed based on the results of the phone survey of similar facilities, discussions with the manufacturer, and recommendations by the District.
- UV Disinfection Building and the LLPS would operate from March 1 to November 30 each year.
- Annual UV lamp replacement and disposal costs were based on the following replacement schedule:
 - o Lamps replaced each year (100% per year)
 - o Ballasts replaced every five years (20% per year)
 - Quartz sleeves replaced every 10 years (10% per year)
 - Wipers replaced every 3 years (33% per year)
 - Lamp disposal costs are included in the costs of the new lamps
- Miscellaneous parts and supplies assumed to be 5% of equipment costs including pumps, valves, piping, HVAC equipment, electrical equipment, etc. UV equipment not included.
- Labor rates were developed based on the data received from the District.
- The labor requirements presented in **Table 10.2-1** were assumed for the three components of the facilities.

Table 10.2-1 - M&O Labor Requirements

Activity	Labor Type	Number	Hours per Week per Worker
Site Work			
Routine Maintenance (Gates,	Laborer	1	10
Roads, Conduit, Utilities,			
Landscaping)			
Low Lift Pump Station			
Routine Maintenance (Pumps,	Laborer	1	10
Valves, Electrical Equipment)	Electrician	1	5
Operations	Operator	1	40
UV Disinfection Building			
Routine Maintenance	Electrician	1	2
Lamp Replacement	Electrician	2	8
Lamp Inspection/Cleaning	Electrician	2	40
Operations	Operator	2	40

10.3 Basis of Net Present Value Calculation

In order to develop a net present worth valve for comparison to other alternatives with differing M&O costs, a present worth factor of 23.17 was used for all present worth calculations, based on a nominal 4.875% interest rate for 20 years with a 3.0% inflation factor.

The interest rate is the 2007 nominal discount rate published by authority of the Water Resources Development Act of 1974. The use of this discount rate mirrors the United States Army Corps of Engineers policy related to calculation of life cycle costs for comparative analysis. The current annual rate can be obtained from the US Department of Agriculture, Natural Resources Conservation Service (http://www.economics.nrcs.usda.gov/cost/priceindexes/rates.html).

The inflation rate was developed by comparison of three common inflation indicators. Those indicators are:

- 1. Gross Domestic Product Deflator
- 2. Consumer Price Index (CPI)
- 3. Producer's Price Index (PPI)

As of the end of August 2007 (most recent available data), the three indicators have a 10-year rolling average inflation of 2.6%, 2.9%, and 2.6% respectively. Data for the GDP Deflator is available from the US Department of Commerce, Bureau of Economic Analysis, Table 1.1.9 (http://www.bea.gov/national/nipaweb/SelectTable.asp). Data for the CPI and PPI is available from the US Department of Labor, Bureau of Labor Statistics (http://www.bls.gov/home.htm). Therefore, a value of 3.0% was selected to provide a reasonable, yet conservative, estimate of inflation.

10.4 Discussion of Cost Estimate Line Items

The preliminary opinion of probable construction cost was developed based on the drawings developed as part of this study (see Volume 2), CTE's knowledge of local construction market, CTE's experience with similar projects, specific budgetary quotes from equipment suppliers, and industry standard practices. The quantities for each item included in the cost estimate were measured from the drawings or estimated based on CTE's understanding of probable means and methods of construction.

In general, unit costs for each line are considered assembly costs including labor and materials for that item plus ancillary items normally associated with that item unless included elsewhere. For example, concrete costs are given including formwork, rebar, and concrete, but not including excavation and backfilling, which are included as separate line items. While an explanation of all line items included in the estimate is not provided, specific line items that warrant additional information are described below in **Table 10.4-1**.

Table 10.4-1 – OPCC Selected Line Item Description

line Ham	Decembrical Additional Information
Concret Requirements	Description/Additional Information
General Requirements	General requirements include project specific insurance (such as payment and performance bonds) and other project specific overhead costs (i.e. field personnel labor, field trailers, field office supplies, general quality control testing, shop drawing preparation, O&M manual preparation, and permit fees). It is assumed to be 15% of the total project direct costs.
Sheeting/Shoring (Site Work)	Cost for installation of sheeting for work on rail embankment including installation by vibratory pile driver on weekends only.
Hand Mining/Connection/ Bulkheading at U/S Connection to Existing Final Effluent Conduit	This line item is a lump sum estimate of the cost to make the connection to the existing final effluent conduit upstream of Gate Structure #1 including hand mining, shoring, demolition, bulkheading, restoration, and backfilling with substantial costs for overtime due to the critical nature of the connection and the need to minimize shutdown of the CTA rail operation. No costs for alternate CTA transportation are included.
Bulkheading and Removal at Gate Structure #3	This line item is a lump sum estimate of the cost to make the connection to the existing final effluent conduit at Gate Structure #3 including demolition, dewatering, bulkheading, restoration, and backfilling.
Utility Items (Site Work)	Assembly costs for utility line items include trenching, shoring, materials, installation, backfilling and placement of topsoil per linear foot of the utility.
Conduits (Site Work)	Assembly costs for conduit line items include excavation, shoring, formwork, rebar, concrete, backfilling and placement of topsoil per linear foot of the conduit.
Pile Mobilization, Piles, Pile Load Test	Costs for installation of 12" concrete piles to support LLPS, UV Disinfection Building, and gate structures. Assumed depth of piles is 50 feet to reach hardpan or bedrock.
Concrete (Base Slabs, Walls, and Elevated Slabs)	Assembly costs for concrete installation including rebar, formwork, and concrete. Does not include excavation or backfill.
Interior walls (masonry)	Assembly costs for construction of masonry interior wall including block, mortar, installation and ancillary costs. Does not include coatings.
Exterior walls (masonry)	Assembly costs for construction of masonry exterior wall including block, insulation, brick, mortar, installation and ancillary costs. Does not include coatings.
Pumps	Budgetary equipment costs from suppliers plus 25% for installation. Includes delivery, startup, and training services.
UV Reactors	Budgetary equipment costs from supplier plus 15% for installation. Includes delivery and installation certification services. Startup and M&O training included separately.
Escalation	Escalation is assumed to be 5% per year. Construction period is assumed to be 35 months. Therefore, escalation to the mid-point of construction is 7.5%.
Contractor's Markup on Subcontractors	Contractor's markup on subcontractors is assumed to be 5%. This markup is applied to all direct project costs except the general conditions line item.
Contractor's Overhead and Profit	Contractor's overhead of 5% includes general contractor overhead including front office costs and project manager's time. Profit is assumed to be 10%.
Contingency	Consistent with AACE guidelines and District policy, a contingency factor of 30% has been added to the OPCC to cover unknown costs associated with the project. Contingency does not include escalation from the point of time of estimate to beginning of construction, extraordinary events, or changes to the scope of the project.

11.0 NSWRP SCHEDULE OF IMPLEMENTATION

The anticipated schedule for implementation of the design of North Side WRP UV Disinfection Facilities is approximately 8 years, assuming no delay between activities after commencement of Preliminary Design. **Table 11.0-1** provides a summary of the anticipated activities and their durations.

Activity	Duration	
Design of Pilot Facility and Concurrent Collimated Beam Testing	12 months	
Program		
Regulatory Review of Pilot Facility Design	6 months	
Construction of Pilot Facility	9 months	
Operation of Pilot Facility/Evaluation of Technology and Scale-up	18 months	
Design of Full-Scale Facility	12-18 months	
Regulatory Review of Full-Scale Design	6 months	
Construction of Full-Scale Facility	24-30 months	
Commissioning/Startup	3 months	
TOTAL	90-102 months	

Table 11.0-1 – Projected Schedule of Implementation

12.0 CALUMET WRP PRELIMINARY COST OPINION

The proposed UV Disinfection Facilities at the CWRP are essentially equivalent to those proposed at the NSWRP. While an evaluation to the level equivalent to the evaluation of the proposed facilities at NSWRP was not completed as part of this study, it can be assumed that the costs for implementation of UV Disinfection at the CWRP would be largely commiserate with the costs for the same facilities at NSWRP. The facilities would be generally identical to the NSWRP facilities with the following exceptions:

- Existing information on soils in the CWRP area indicates that there is no need for deep foundations to support the proposed structures, nor for the upstream and downstream connections to the existing effluent (assumed to reuse existing chlorine contact chamber gates).
- 2. The arrangement of the CWRP facilities, specifically the digesters, chlorine contact chambers (out-of-service), and final effluent conduits create a constrained site with no space available to locate the proposed disinfection facilities. Therefore, it is assumed that the chlorine contact chambers would be demolished to provide space for the UV Disinfection Facilities. However, it is assumed that only the walls would be demolished and the void filled with common fill or structural fill as appropriate. The existing base slab would be left in place.
- The ComEd power transmission system improvements are based on the transmission system specific to CWRP. Per ComEd representatives, the only expected improvements would be adjustment to existing protective devices at the ComEd transformer yard at CWRP.
- 4. The peak design flow for CWRP is 480 mgd compared to 450 mgd for the NSWRP.

It should also be noted that the CWRP is currently experiencing restrictions in plant capacity during wet weather events due to backwater effects in the plant outfall. Therefore, a LLPS is assumed necessary at the CWRP.

To estimate the costs for the UV Disinfection Facilities at CWRP, CTE deducted the costs for the deep foundations and special connections to effluent conduits, multiplied the remaining capital cost estimate by the ratio of 480 mgd to 450 mgd, and added the cost for demolishing the existing chlorine contact chambers. **Table 12.0-1** provides a summary of those actions.

Table 12.0-1 – Summary of CWRP UV Disinfection Facility Cost Development

NSWRP Site Work	\$27,170,000		
DEDUCT Deep Foundations	\$5,760,000		
Subtotal	\$21,410,000		
MULTIPLY by Ratio of Flows	1.067		
Subtotal	\$22,840,000		
ADD Chlorine Contact Chamber Demolition	\$4,980,000		
CWRP Site Work	\$27,800,000		
NSWRP LLPS	\$27,010,000		
MULTIPLY by Ratio of Flows	1.067		
CWRP LLPS	\$28,800,000		
NSWRP UV Disinfection	\$49,490,000		
MULTIPLY by Ratio of Flows	1.067		
CWRP UV Disinfection	\$52,800,000		
CWRP Total	\$109,400,000		

Table 12.0-2 provides the preliminary opinion of probable construction cost for the UV Disinfection Facilities at the Calumet Water Reclamation Plant. Annual M&O costs are also provided assuming the same labor costs as NSWRP but increased energy, parts, and supplies costs per the same ratio as used for the capital costs.

Table 12.0-2 - CWRP UV Disinfection Facilities Preliminary OPCC and M&O Costs

Capital Cost Estimates				
ComEd Service Upgrade Charge	\$130,000			
A. General Sitework	\$27,800,000			
B. Low Lift Pump Station	\$28,800,000			
C. Disinfection System	\$52,800,000			
Total Capital Cost	\$109,530,000			
Maintenance & Operations Cost Estimates				
A. General Sitework	\$130,000/yr			
B. Low Lift Pump Station	\$890,000/yr			
C. Disinfection System	\$3,490,000/yr			
Total Annual M&O Cost	\$4,520,000/yr			
Total Present Worth M&O Cost	\$104,600,000			
Total Present Worth	\$214,100,000			

All costs in 2007 dollars.

Volume 2 of the Cost Study Report for

DISINFECTION COST STUDY UV DISINFECTION FACILITIES

NORTH SIDE WATER RECLAMATION PLANT Chicago, Illinois

Contract 07-026-2P



Room 508, 100 East Erie Street

Chicago, Illinois 60611

Board of Commissioners

2008

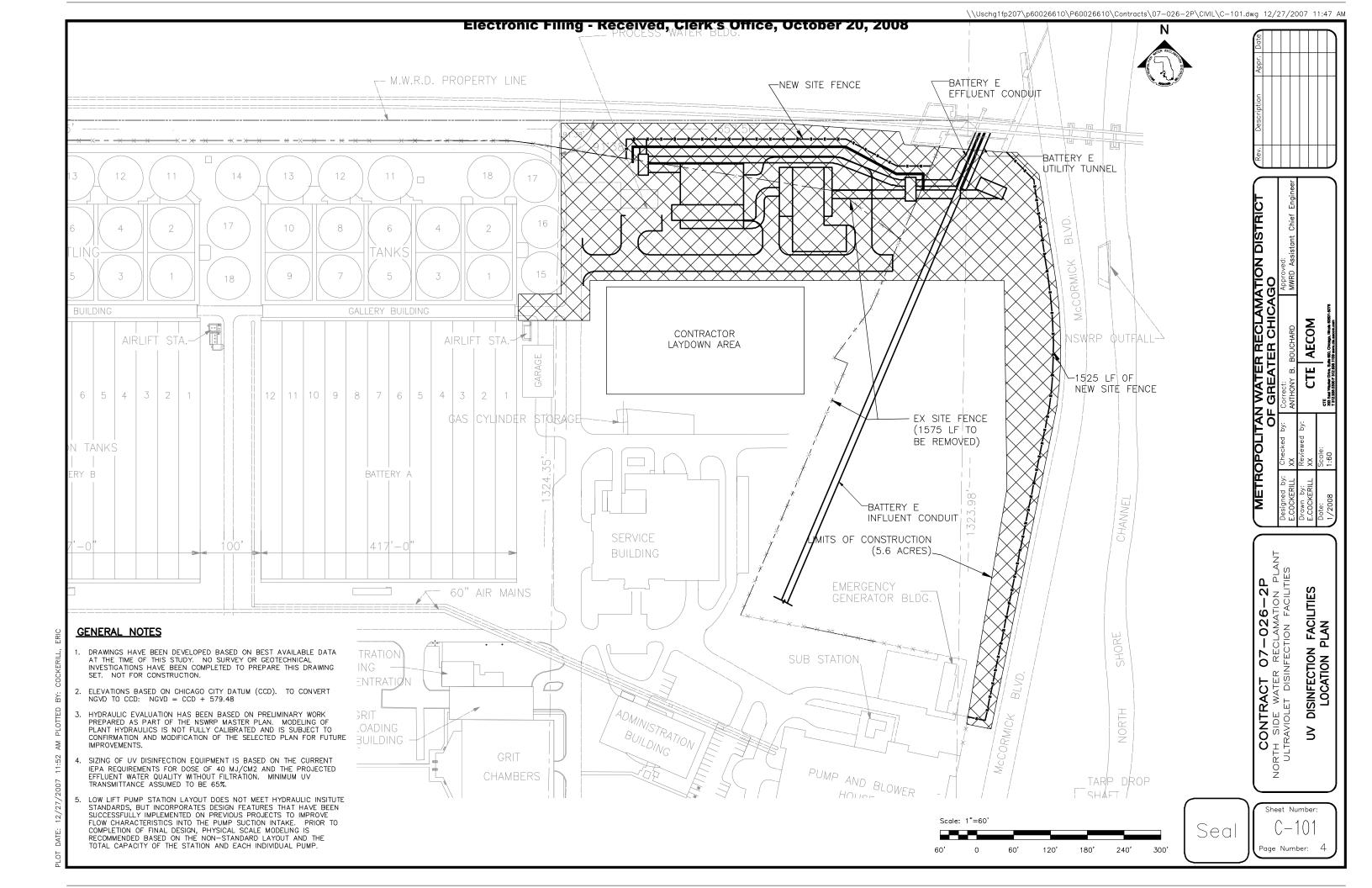
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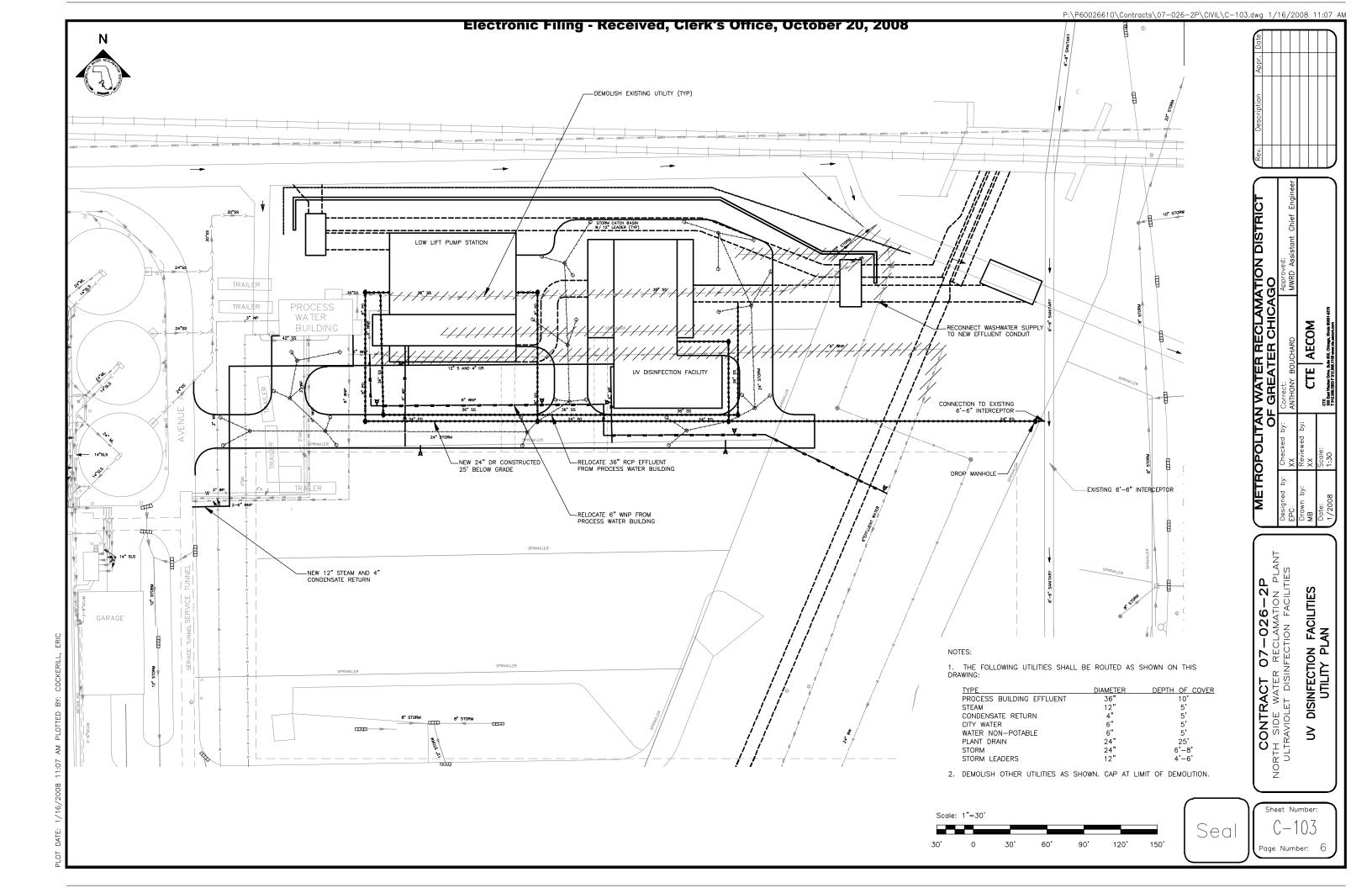
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Darlene A. LoCascio	Purchasing Agen
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Frederick M. Feldman	Attorne
Patrick J. Foley	Director of Personne
Keith D. Smith	Director of Information Technolog

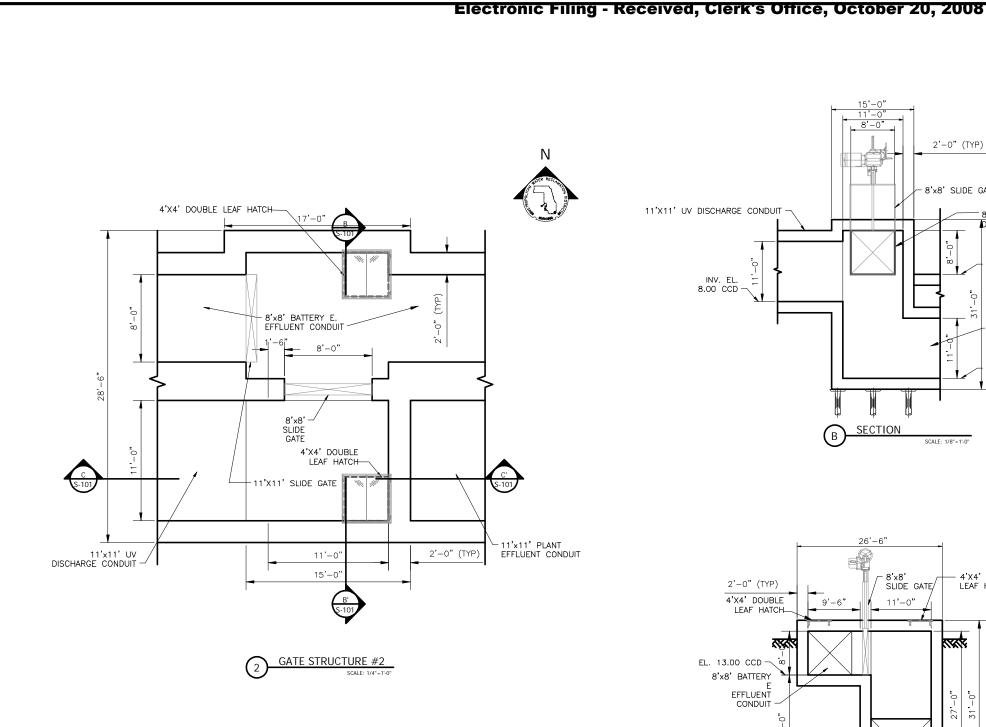
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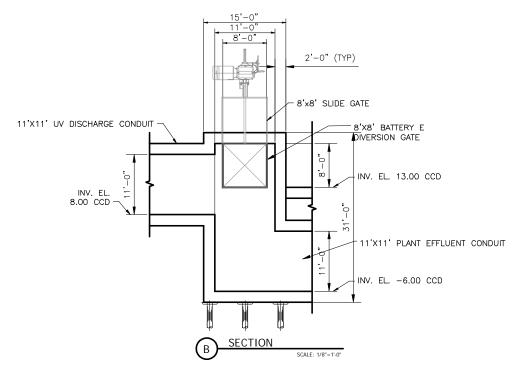
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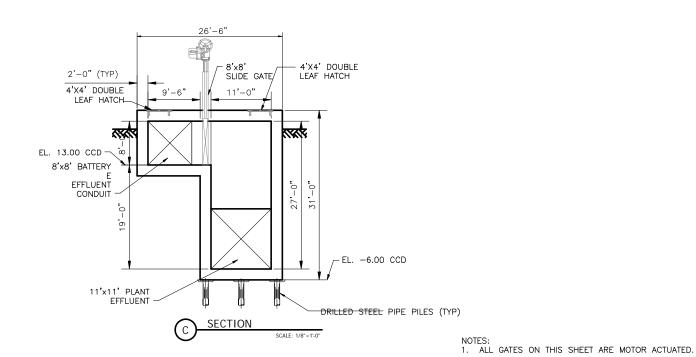
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12	S-202	-	UV DISINFECTION FACILITY ROOF FRAMING PLAN	36	P-303	-	LOW LIFT PUMP STATION SECTION
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14	S-301	-	LOW LIFT PUMP STATION FOUNDATION PLAN	37	M-001	-	MECHANICAL LEGEND AND ABBREVIATIONS
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			ARCHTECTURAL DRAWINGS	40	E-001	-	ELECTRICAL LEGEND
18	A-201	-	UV DISINFECTION FACILITY PLANS	41	E-002	-	ELECTRICAL ABBREVIATIONS
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METROPOLITAN WATER RECLAMATION DISTRICT OF GREATER CHICAGO CTE AECOM

> #2 STRUCTURE SECTIONS

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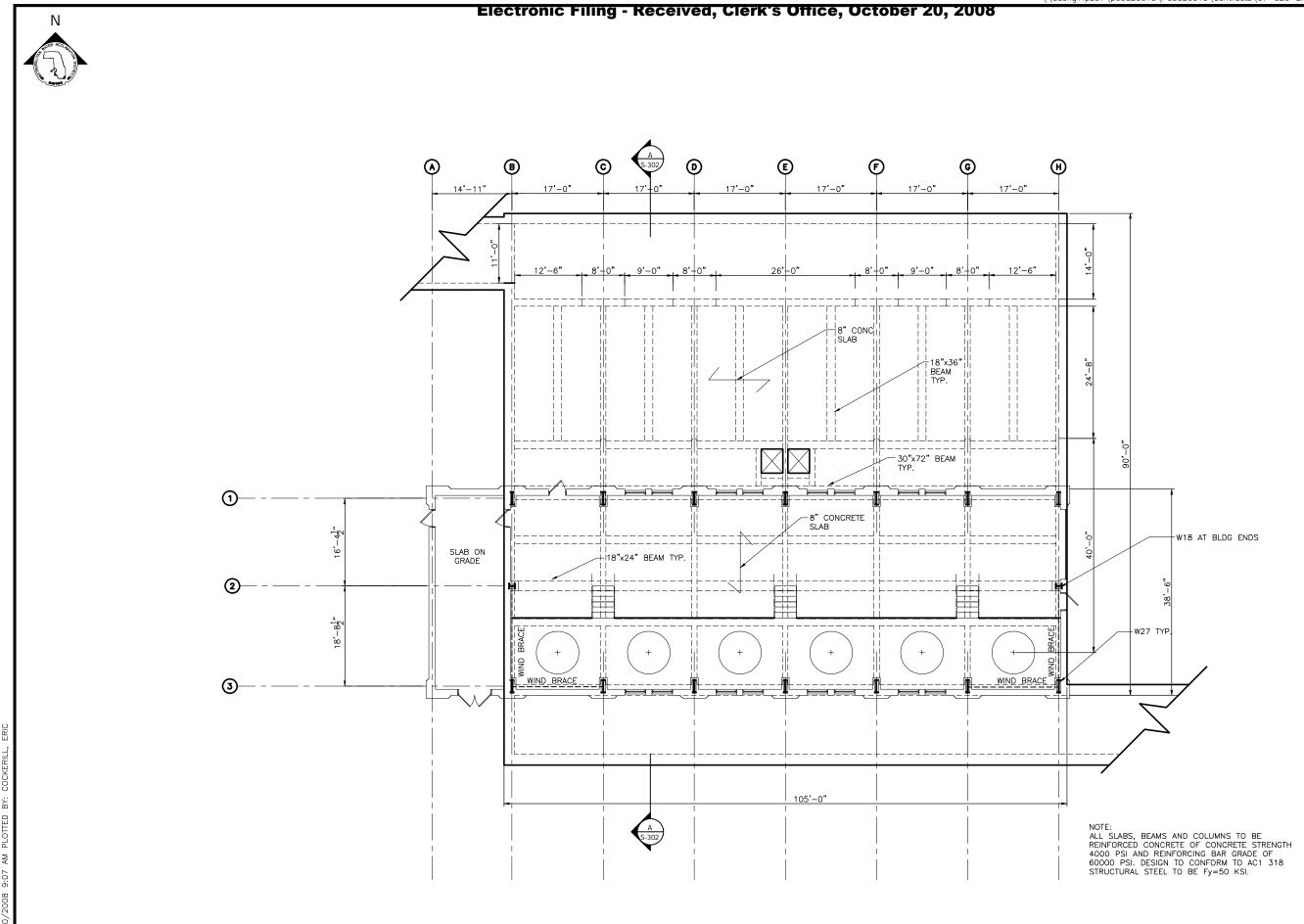
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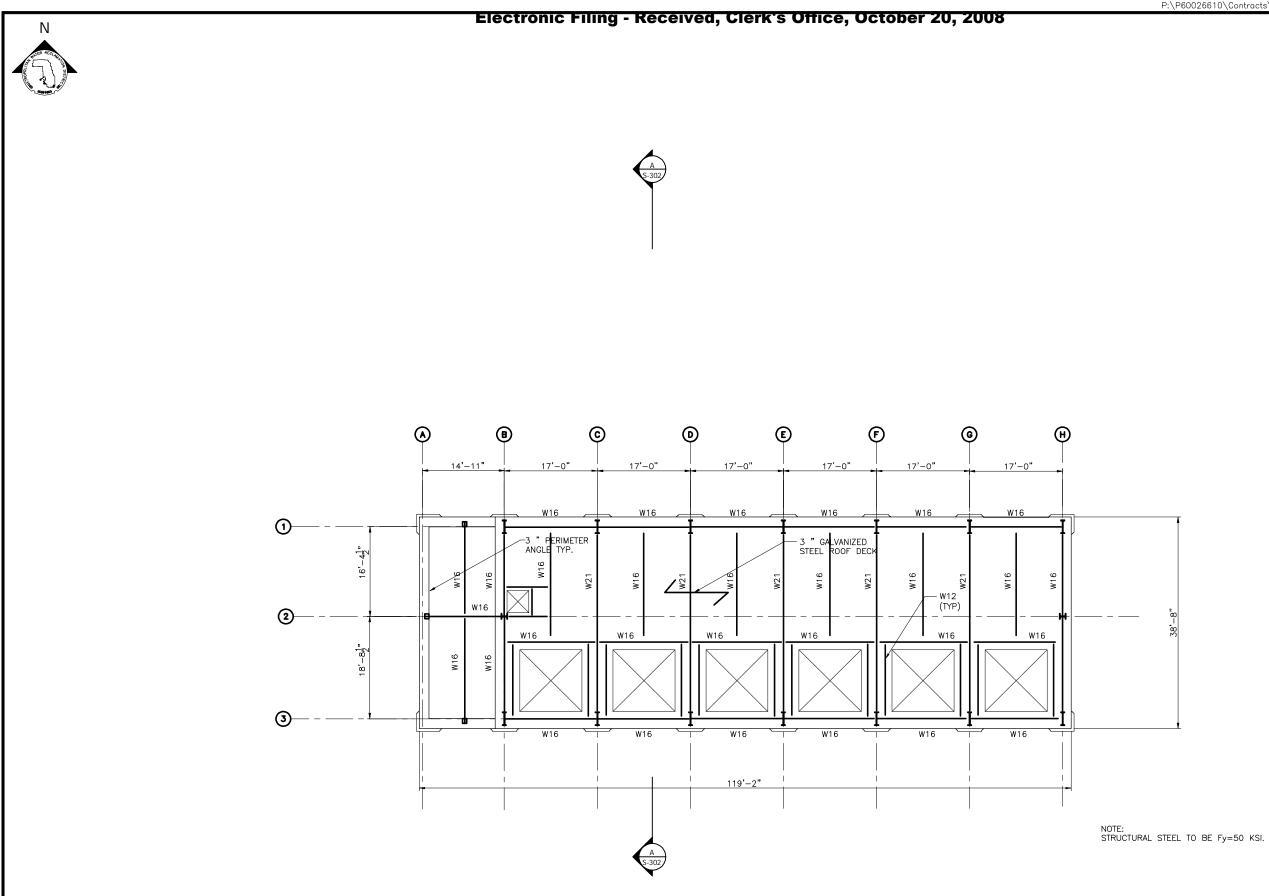


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LOW LIFT PUMP GRADE LEVEL

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OF GREATER CHICAGO
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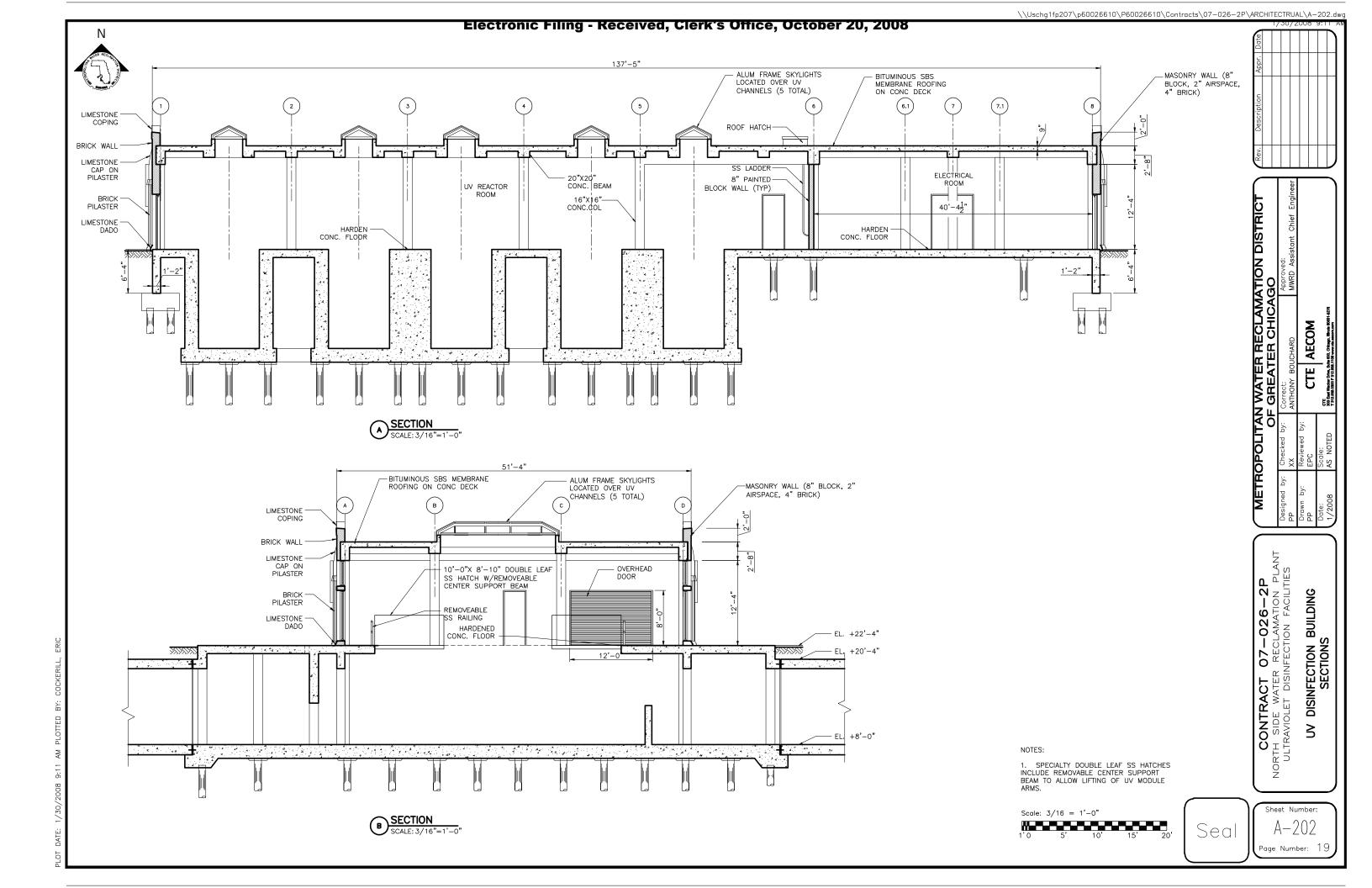
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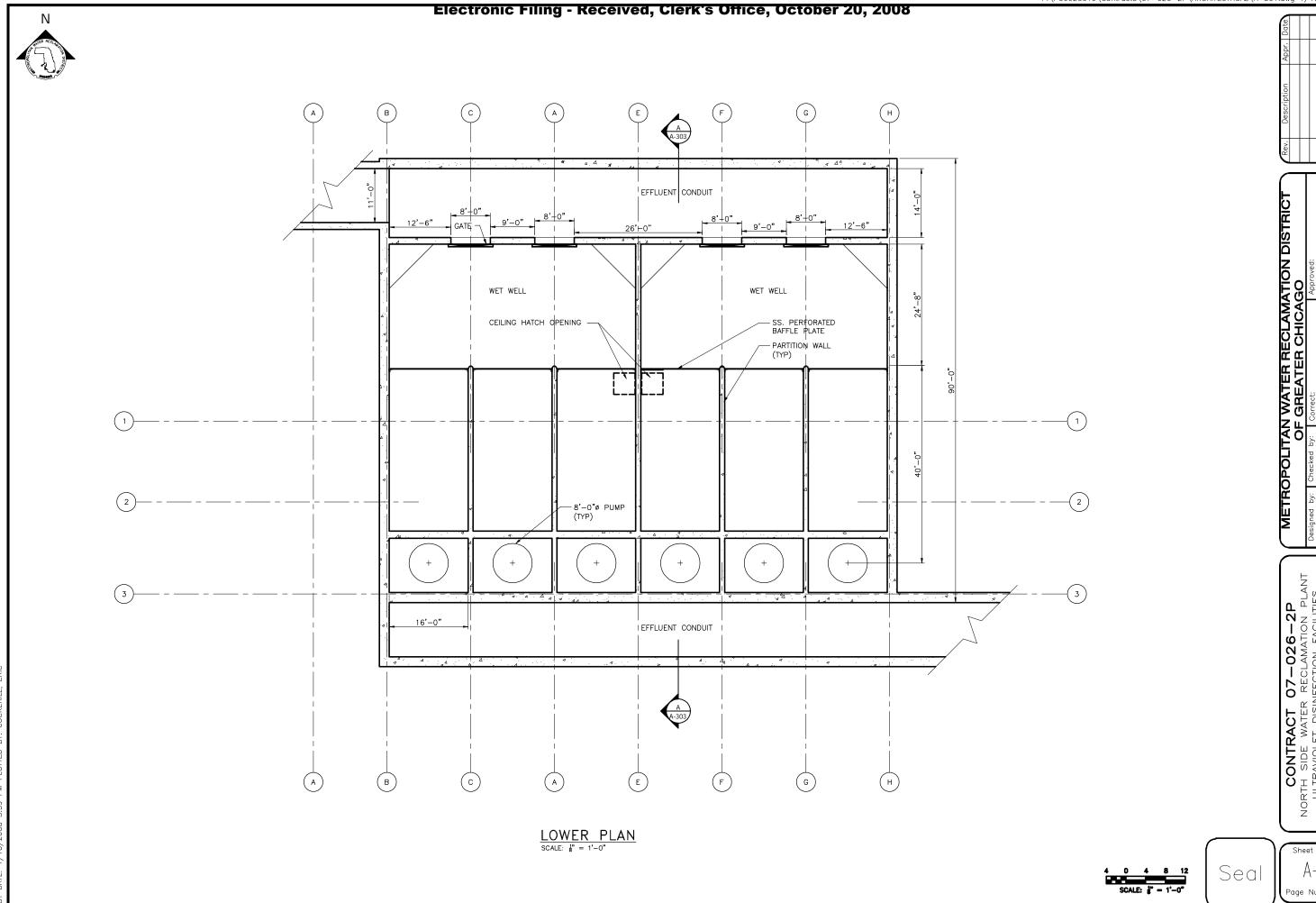
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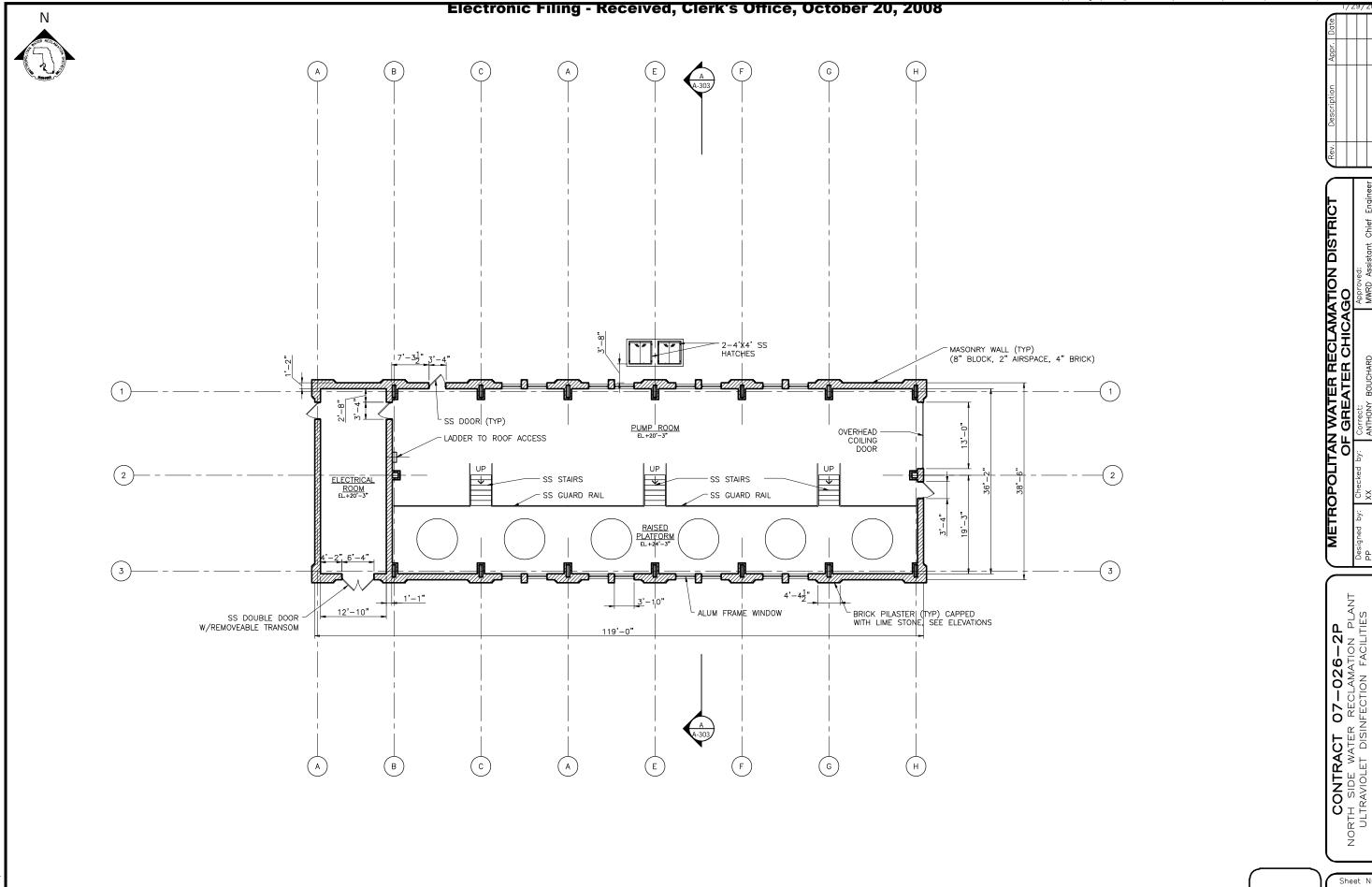




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NORTH SIDE WATER RECLAMATION PLANT
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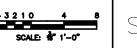
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LOW LIFT PUMP STATION UPPER PLAN

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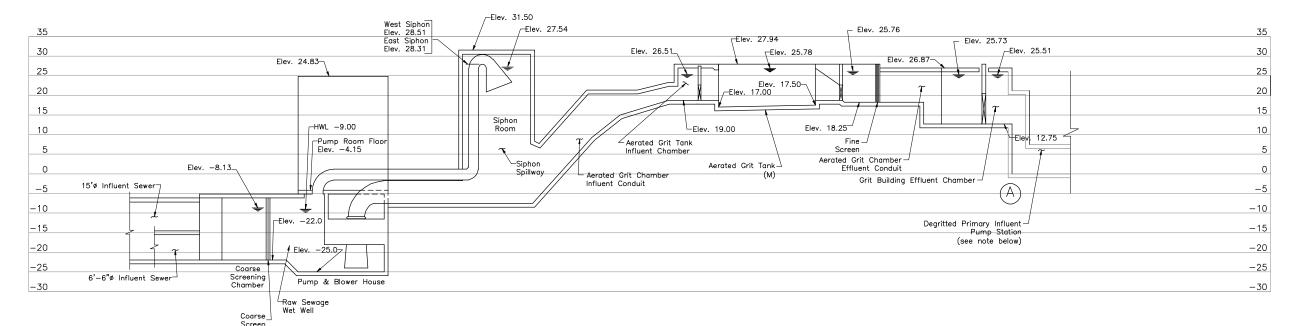
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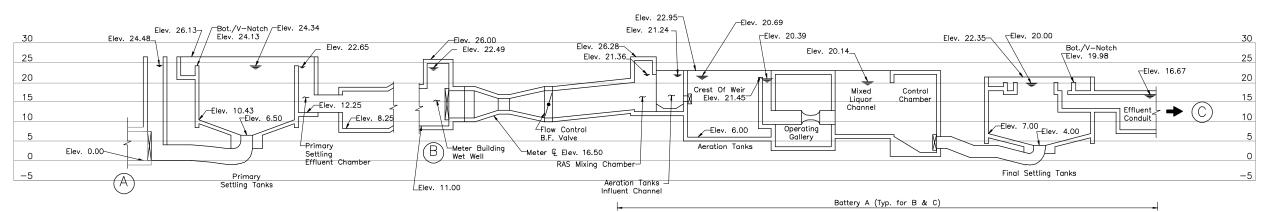


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<u>Legend</u>

Flow diversion from Headworks to Batteries on Existing Site and North Site Battery E. (Existing Site - 345 mgd, Battery E - 105 mgd)

B) - Flow split at Meter Building Wet Well to Batteries A & F

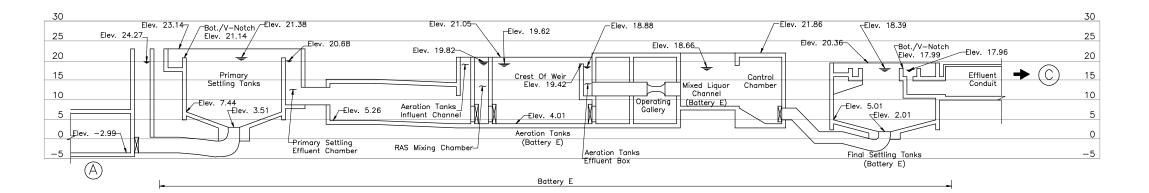
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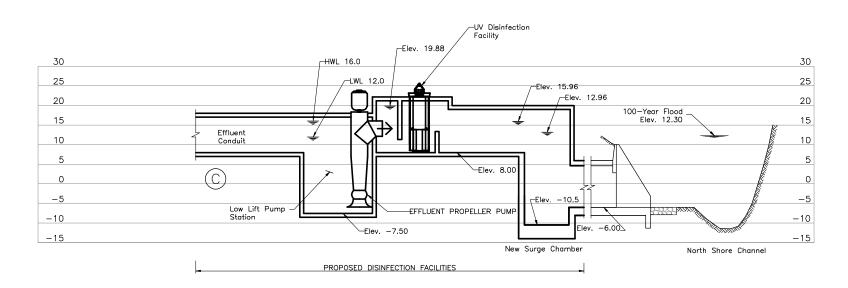
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HYDRAULIC I A, B, C, D,

PROPOSED BATTERIES





<u>Legend</u>

(A) - Flow diversion from Headworks to Battery E [Q (Battery E) = 105 mgd]

© - Flow junction from Batteries A, E, & F. (Total Q = 450 mgd)

METROPOLITAN WATER RECLAMATION DISTRICT

OF GREATER CHICAGO

Designed by: Checked by: Correct: ANTHONY BUCHARD ANTHONY BUCHARD

CONTRACT 07-026-2P

NORTH SIDE WATER RECLAMATION PLANT

ULTRAVIOLET DISINFECTION FACILITIES

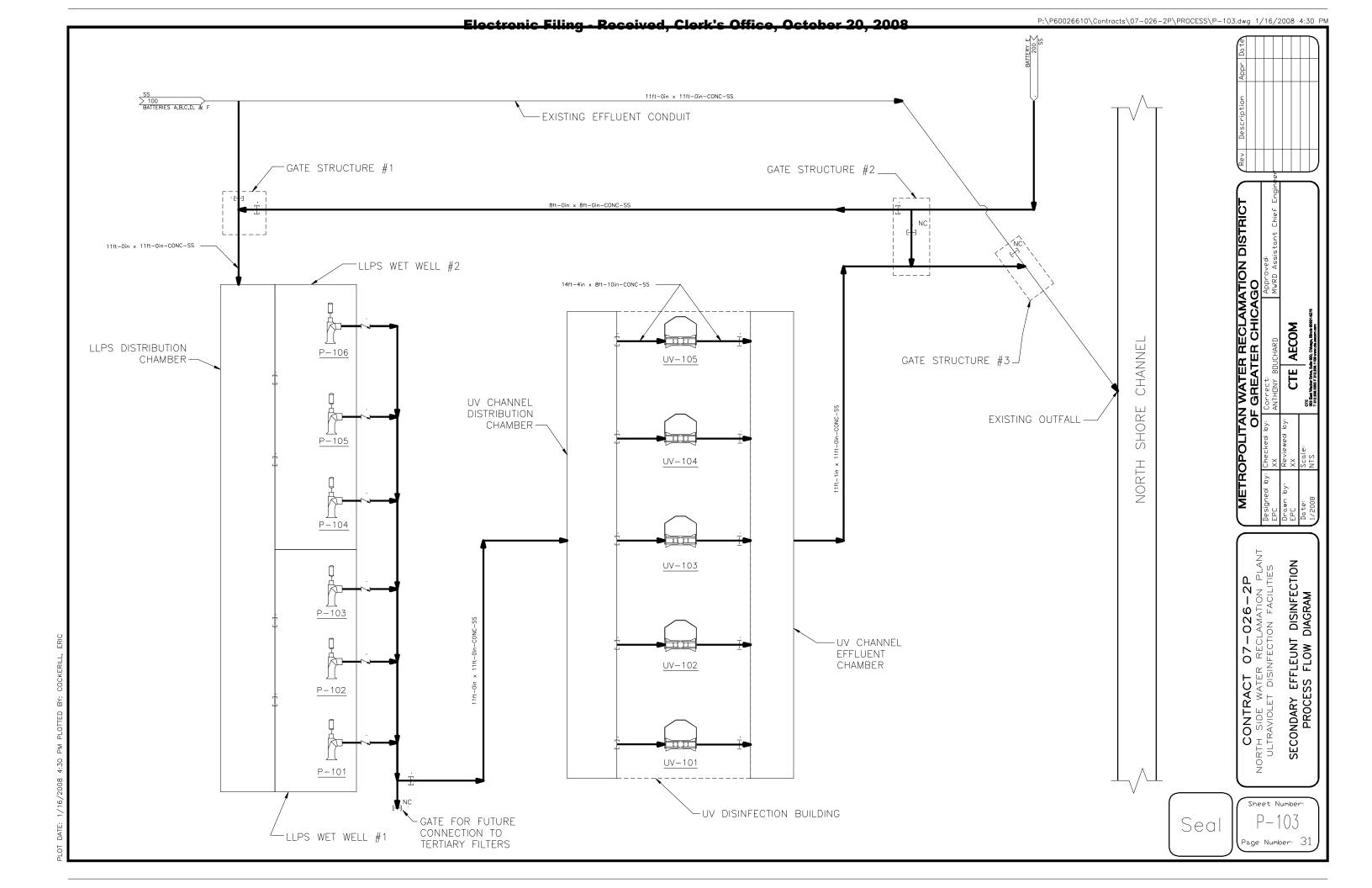
PROPOSED HYDRAULIC PROFILE

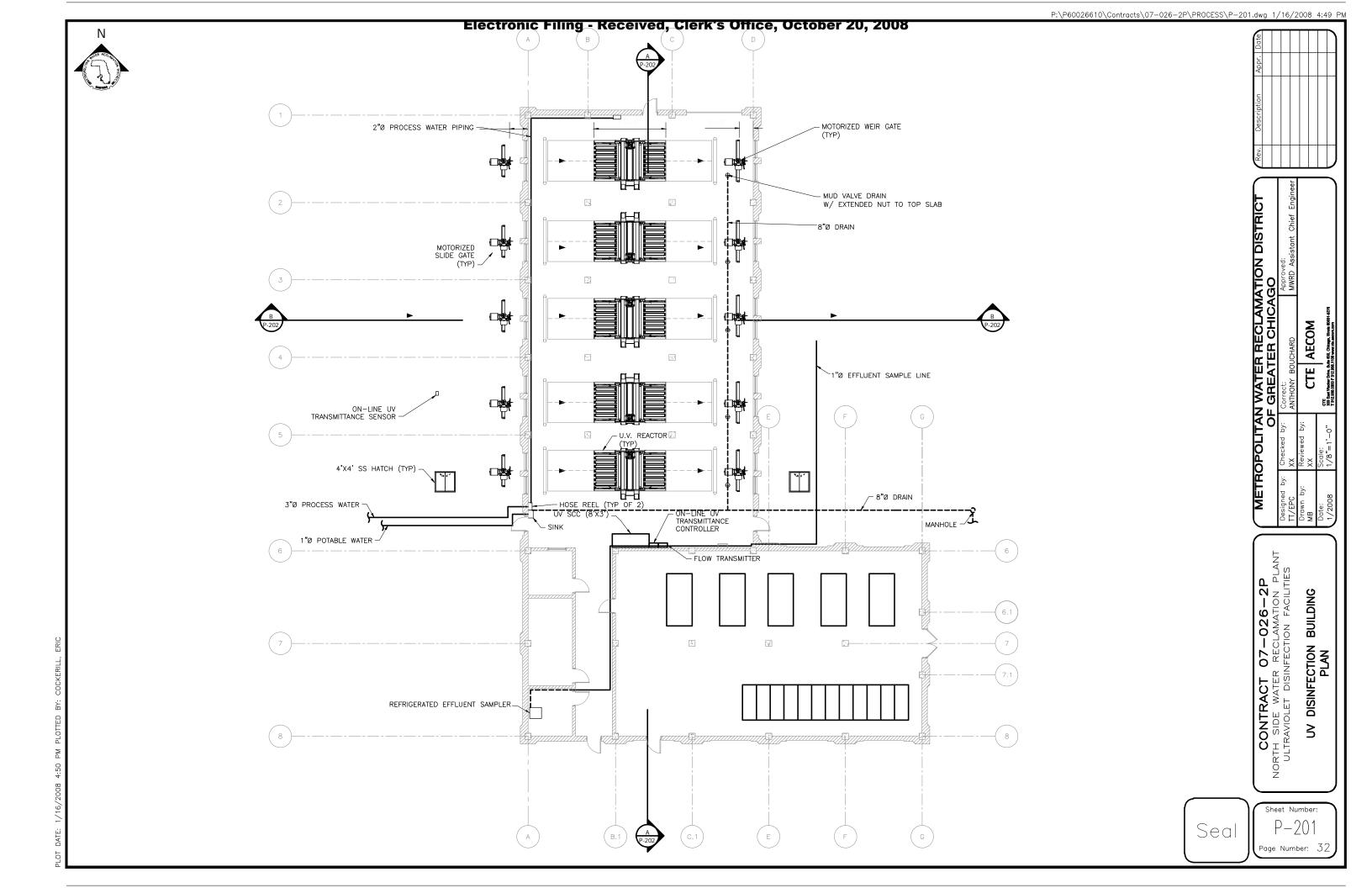
BATTERY E AND DISINFECTION FACILITIES

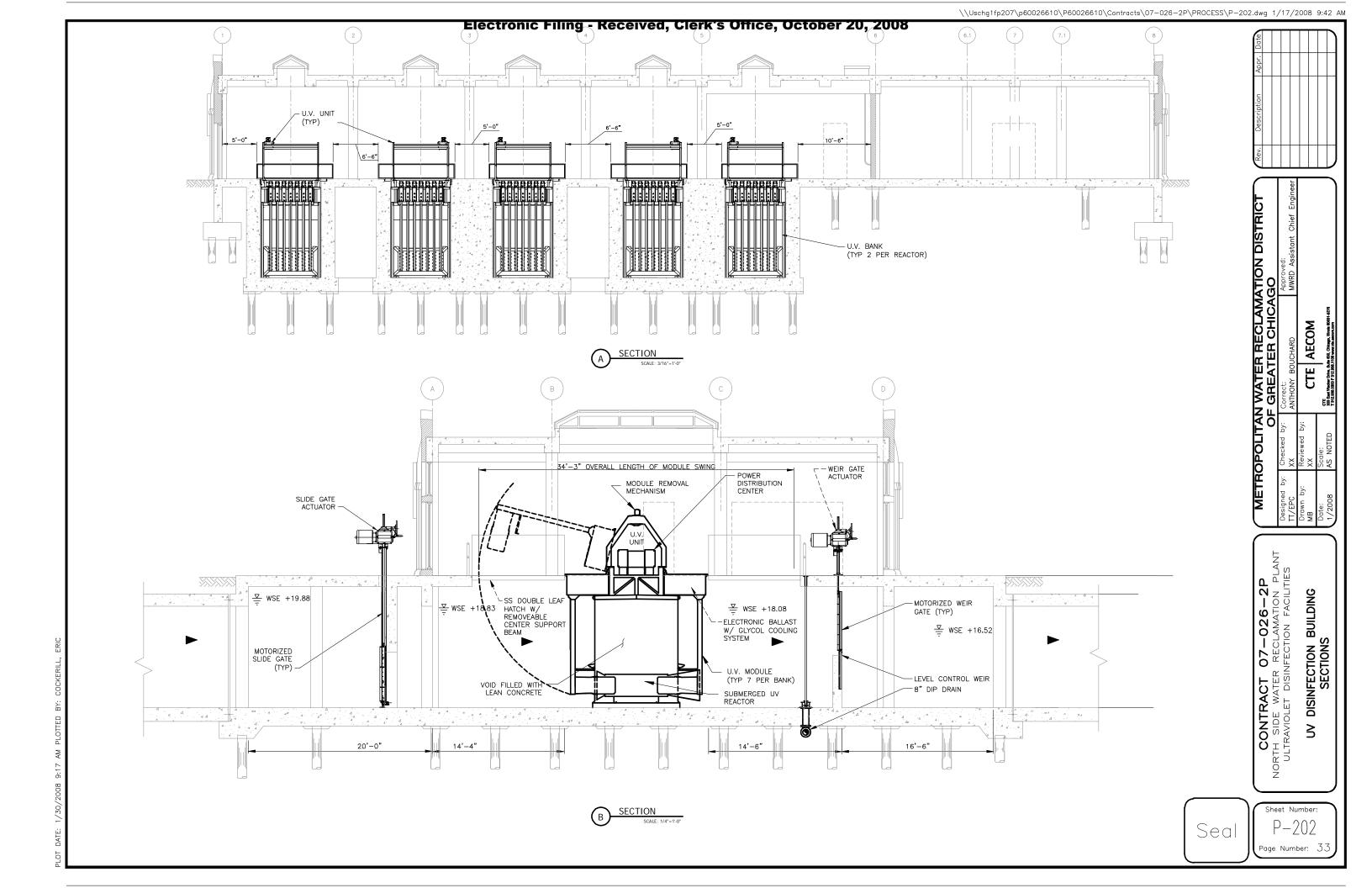
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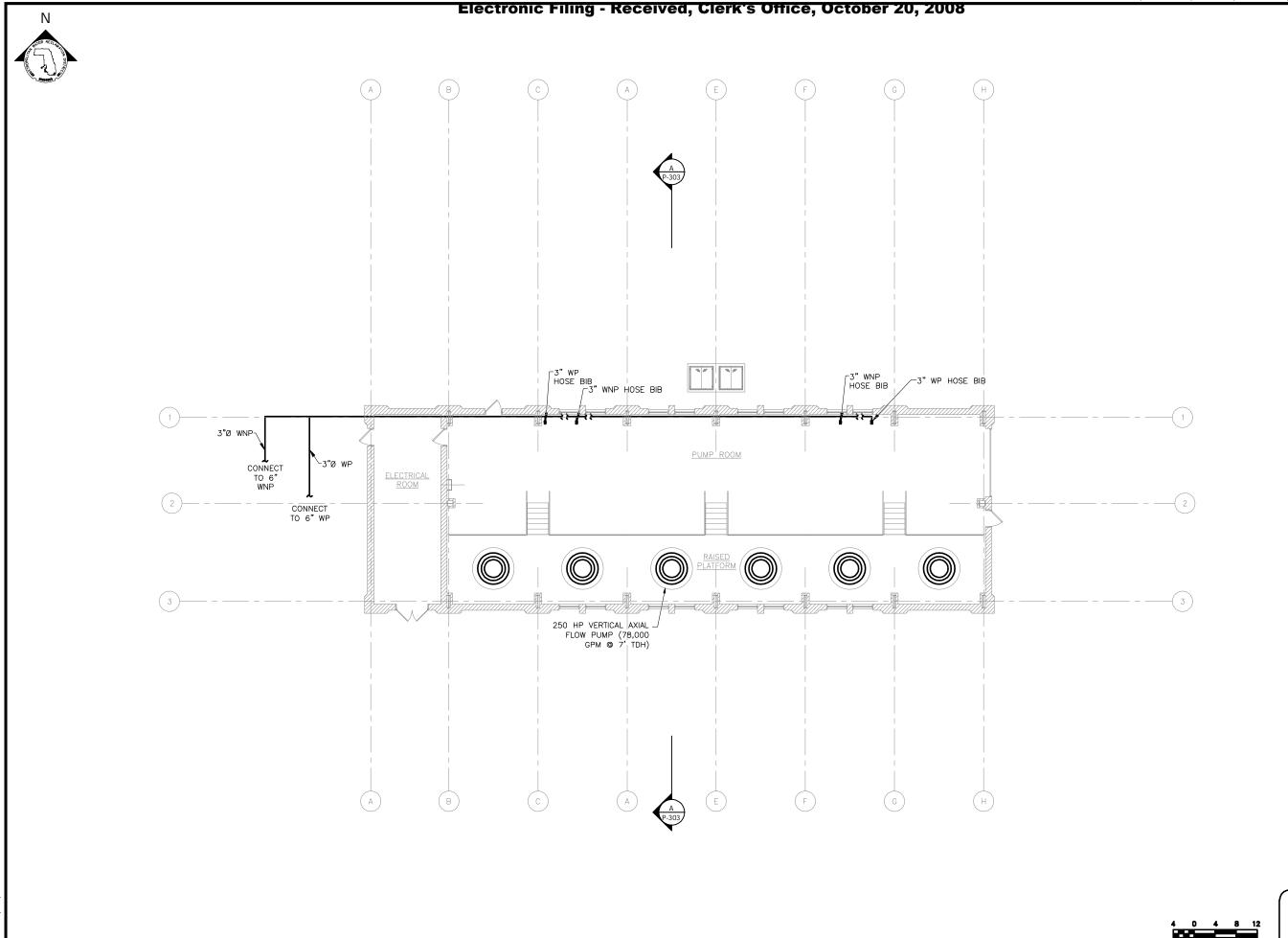


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8"ø DRAIN -

8" PLUG VALVE -



METROPOLITAN WATER RECLAMATION DISTRICT OF GREATER CHICAGO CTE AECOM CONTRACT 07-026-2P NORTH SIDE WATER RECLAMATION PLANT ULTRAVIOLET DISINFECTION FACILITIES

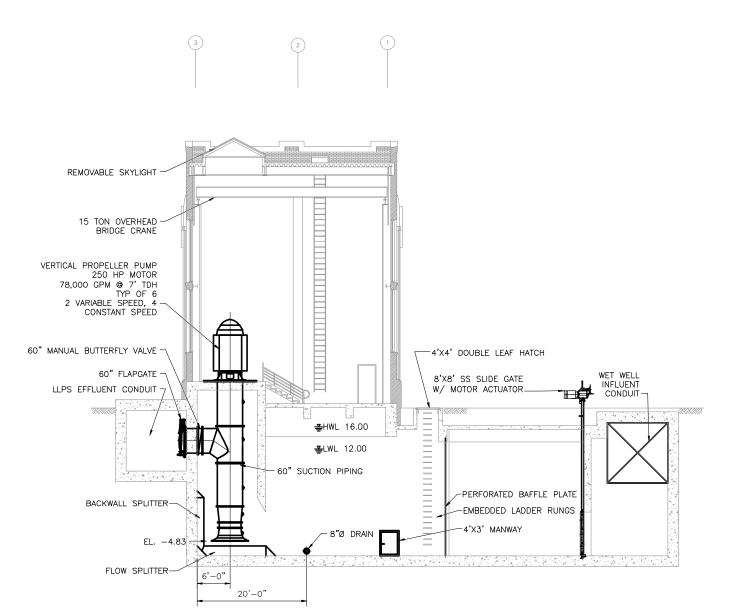
STATION PLAN

LOW LIFT PUMP : UPPER LEVEL

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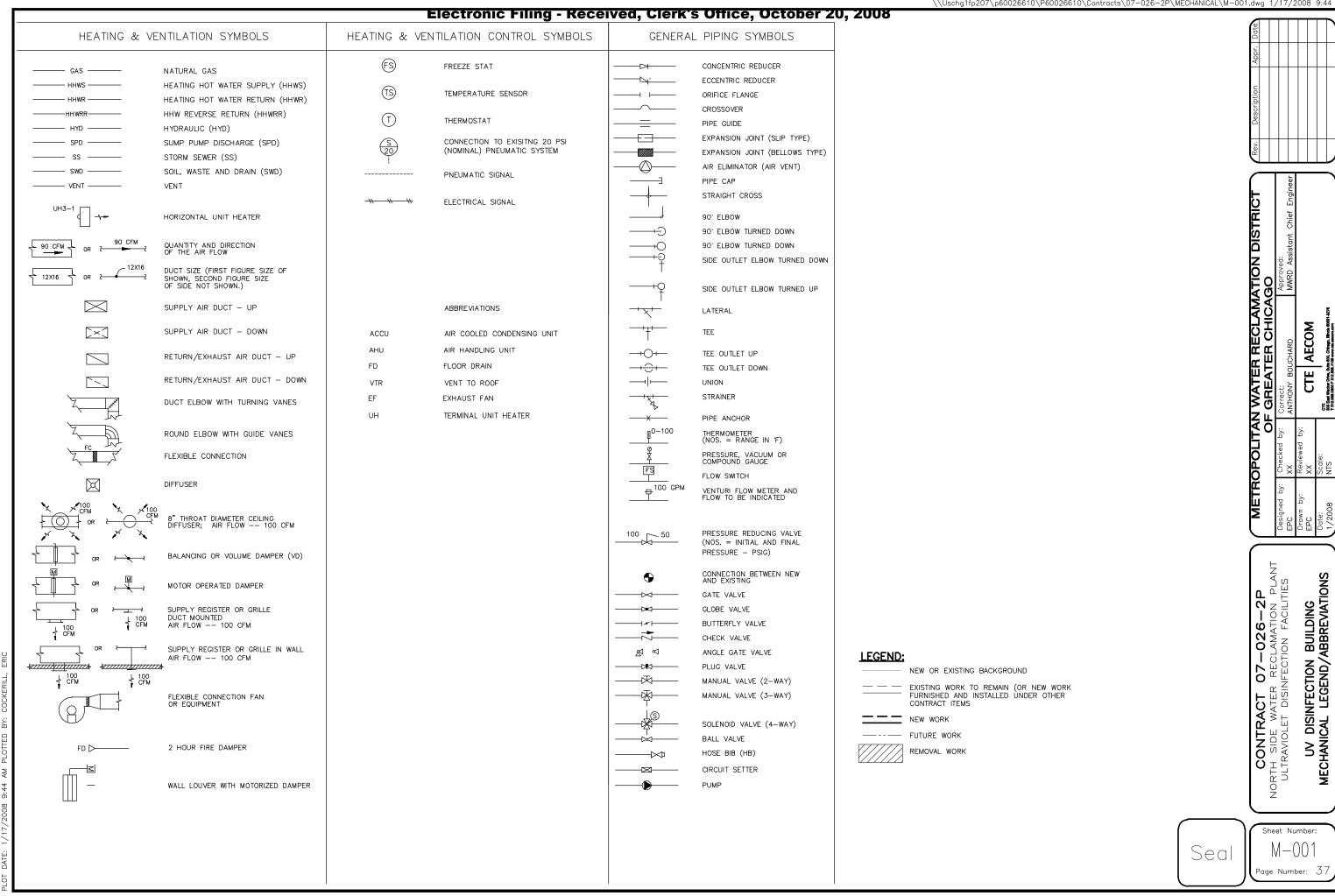


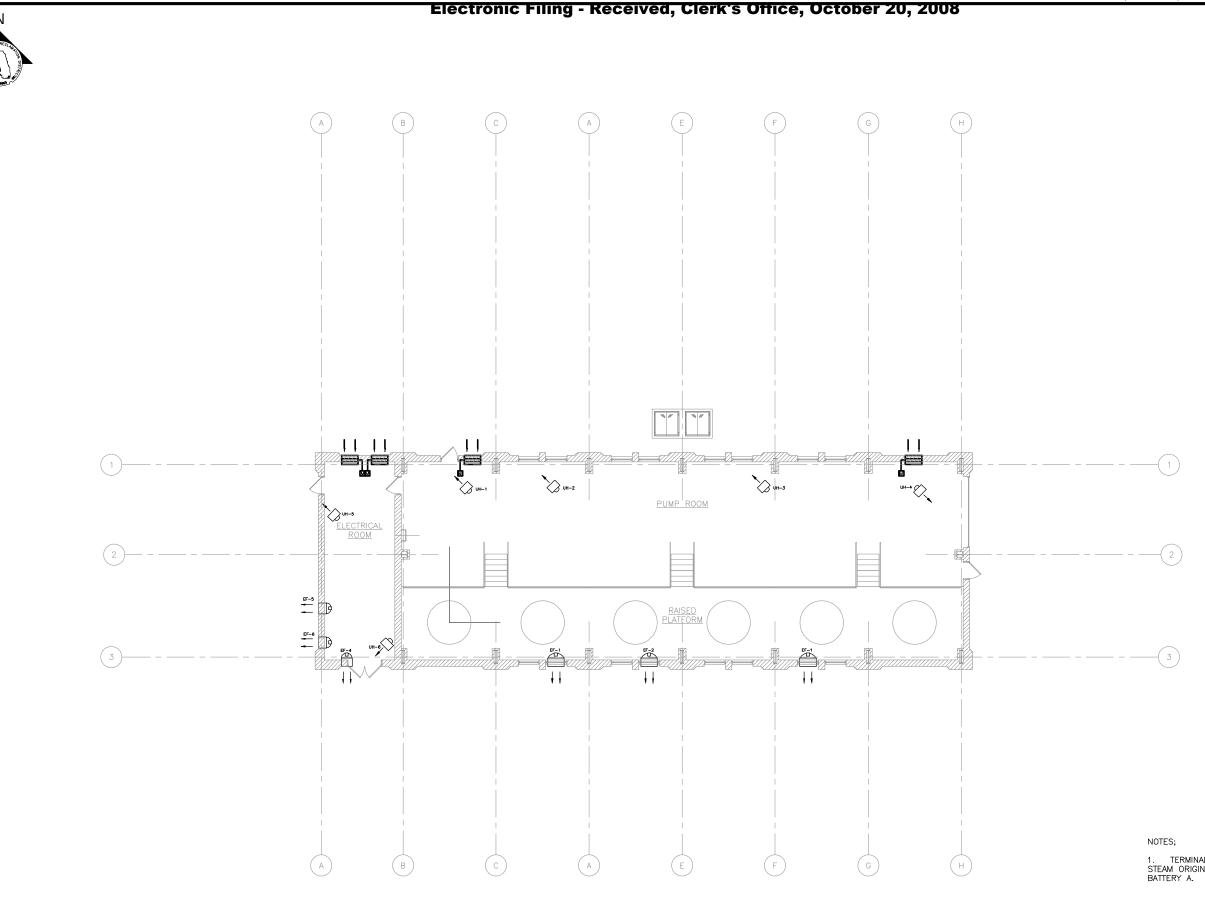
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METROPOLITAN WATER RECLAMATION DISTRICT OF GREATER CHICAGO CTE AECOM

CONTRACT 07-026-2P NORTH SIDE WATER RECLAMATION PLANT ULTRAVIOLET DISINFECTION FACILITIES STATION

LOW LIFT PUMP S SECTION





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SINGLE LINE DIAGRAMS

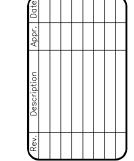
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YMBOLS	DESCRIPTION	SYMBOLS	DESCRIPTION
—	Single Arm Pole Mounted Incandescent Or High Intensity Discharge Fixture As Shown In Fixture Schedule	\$ +48	Single Pole Taggle Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
M	Twin Arm Pole Mounted Incandescent Or High Intensity Discharge Fixture As Shown In The Fixture Schedule	\$ +48 2	2 — Pole Toggle Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
$\bowtie\!$	Bracket Mounted Incandescent Or High Intensity Discharge Fixture As Shown In Fixture Schedule	\$ +48 3	3 — Pole Toggle Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
'LPX-X'	Incandescent Or H.I.D Type Fixture As Shown In Fixture Schedule. See Note No. 3.	\$ +48 4	 4 - Pole Taggle Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
'LPX-X' F O	Fluorescent Lighting Fixture As Shown In Fixture Schedule. See Note No. 3.	\$T+48	Manual Motor Starter Switch W/Overload Protection And Enclosure (Number Indicates Mounting Height, If No Number Is Shown, See Specifications.)
⊗ SFI⊝ +	Exit & Directional Sign As Shown In Fixture Schedule Duplex GFI Receptacle (number Indicates Mounting	\$ _D	Single Pole Dimmer Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
0F 18 ⊖—1	Height. If No Number Is Shown, See Specifications) Single Convenience Receptacle (Number Indicates Mounting Height, If No Number Is Shown, See	\$ _K	Key Operated Switch (Number Indicates Mounting Height, If No Number Is Shown, See Specifications)
18⊖	Specifications) Duplex Convenience Receptacle (Number Indicates Mounting Height, If No Number Is Shown, See	<u></u>	Battery Powered Emergency Lighting Fixture
	Specifications) Welding Receptacle And Enclosed Fusible Disconnect Switch. 3P-60A, 600V, Nema 4, Unless Noted Otherwise.		Unit Heater
48 ④ ──	(Number Indicates Mounting Height, Unless Noted Otherwise.) Crouse-Hinds WSR6351. Special Purpose Explosion Proof Receptacle	H	Alarm Horn
Φ ⊣	As Shown in Fixture Schedule. Duplex Receptacle Installed As Part Of Cabinetry	S	Speaker
•	Or Furniture. Connect As Required By Manufacturer Floor Mounted Duplex Convinience Receptacle With Adjustable Floor Box, Plate, And Carpet Flange	(S)	Smoke Detector (Ionization Type)
▶w ⊳w	Where Required Voice System Outlet. 'W' Denotes Wall Mounted.	D	Rate Of Rise Heat Detector
⊬	Data System Outlet. 'W' Denotes Wall Mounted. Telephone Utility System Outlet Installed As A Part Of Cabinetry Or Furniture. Connect As Required	M	Manual Fire Alarm Pull Station
T	By Manufacturer. Telephone	Щ _Е	Fire Alarm System Horn
	Lighting Panel Remote Telemetry System Cabinet	<u>(G)</u>	Combustible Gas Detector
	Telephone Utility System Distribution Panel	P	Cctv Camera Exposed Conduit
	Cabinet Or Pull Box Unfused Safety Switch, 3P-30A, 600V, In Nema 4		Underground Duct As Noted
Z'	Stainless Steel Enclosure, Unless Otherwise Noted. Fused Safety Switch, 3P—30A, 600V, In Nema 4		Conduit Concealed In Floor Slab Or Under
E r	Stainless Steel Enclosure, Unless Otherwise Noted. Outlet Or Junction Box		Floor Slab. (Conduits 1—1/4" Or Larger Shall Be Installed Under Floor Slab). Conduits Run Under Floor Slab Shall Be Encased In Concrete.
⊠h	Combination Protective Device And Magnetic Starter		Homerun As Indicated In Conduit, Cable And Wire Tabulation. '#### Denotes Conduit Number.
	Single Unit Pushbutton Station 2-Unit Pushbutton Station	'####' 'XXXX'	See Conduit, Cable And Wire Tabulation Sheets For Items Designated With 4 Conduit Number, 'xxxx' Denotes The Panel To Homerun To Conduit 1" Or Smaller May Be Installed In Slab. Homeruns To Panels As Indicated Shall Conform
T	3-Unit Pushbutton Station	~E~	no Note No. 3 Below. See Panelboard Schedules And Plan Sheets For Circuiting.
	Electric Motor — "Number" Indicates Approximate	R	Existing Conduit And Wire Shall Remain Existing Conduit, Wire, Boxes, Etc., Which
	Horsepower, "F" Denotes Fractional Horsepower.	_A~	Shall Be Removed. Existing Conduit Which Shall Be Abandoned.
(GEN) (A) ^R	Generator Alarm Station Including Relay Enclosure And Alarm Beacon With Horn. 'R' Denotes Remote		Disconnect And Remove Existing Conductors. Cut Off Conduit Flush W/Finished Surface And Fill W/Grout.
<u>[5]</u>	Alarm Beacon With Horn. R. Denotes Remote Alarm Beacon With Horn Only. See Electrical Details. "Hand-Off-Automatic" Selector Switch	∠E/R_	Existing Conduit Which Shall Be Reused. Remove Existing Conductors And Install New Conductors As Indicated Or Noted On Plan.
9 ∑	Vacuum Switch	•	Ground Rod
T S	Limit Switch	H	Electrical Handhole Electrical Manhole
T Ş	Flow Switch	[W]	CONDUIT SYSTEM NOTES
I P Ş	Pressure Switch	So Located As No And Shall Be Space	d In Structural Concrete (Floor Slabs, Etc.) Shall Be t To Unduly Impair The Strength Of The Construction sed Not Less Than Two Times The Conduit O.D. Between
і Б Ş	Door Switch	By The Engineer. Shall Be Based Or Slab, Maximum Co	Except Where Crossing Or Otherwise Approved Spacing Between Adjacent Conduits Of Different Sizes in The Larger Conduit. Locate Conduits In Center Of Induit O.D. Size Embedded In Concrete Slob And Walls
T S	Torque Switch	Shall Not Exceed Go Through Beam: Engineer, Spacing	1/5 Of The Slab Or Wall Thickness. Conduit Shall Not s Or Lintels Over Openings Without Approval Of The Of Conduits For Existing Construction Shall Be As
	Pneumatic/Electric Switch	Required Shall Be	New Construction. Any Structural Modifications Done At No Cost To The Owner. ut Further Designation, Indicates 2#12 In 3/4 Inch Conduit.
	Safety Pull Cord	Wiring For Lighting	ut Further Designation, Indicates 2#12 In 3/4 Inch Conduit, Wires Are Indicated As Follows: —/— (3-wires) —// mf— ger Hatchmark Indicates Neutral Conductor. Receptacles And Other Miscellaneous Circuits
i C S	Control Station (See Electrical Details)	Shall Be 2 #12- Indicated On The Arrangement And	3/4" C. (Minimum) And Shall Conform To The Circuiting Drawings With Additional Conductors, Conduits, Routing As Required For A Complete And Functional 3 Shall Be So Arranged That No More Than Six Current
(FS)—	Float Switch	Carrying Conductor	g stail be so Arranged Ind. No wore that six current rs Shall Be Installed Per Conduit And Circuits Of hall Be Installed In Separate Raceways.
(EP)—	Electro-Pneumatic Valve		
<u>\$</u> —	Solenoid Valve		
D —	Electric Thermostat		
(D)—	Electric Damper Motor		

	SCHEMA	ATIC WIRING DIAGRAMS
	SYMBOLS	DESCRIPTION
	——	Contact, Normally Open
		Contact, Normally Closed
	L.O.S.	
	0 0	Pushbutton, Lock Out Stop
	0 0	Pushbutton, Normally Closed
	⊸ •–	Pushbutton, Normally Open
		Selector Switch — "Hand—Off—Auto". Unless Otherwise Noted.
	0 0 0	Pushbutton, Maintained Contact, Double Circuit
	or, s 	Overloads
		Fuse
	>	Switch
		Pilot Light (Push To Test)
	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Manual Motor Starter
	M	
		Auxiliary Starter Contacts
	<u>⊸</u> _	Pressure Switch, Opens On Rise
	~~~	Pressure Switch, Closes On Rise
	— >	Limit Switch, Normally Closed
		Limit Switch, Normally Open Limit Switch, Normally Open, Held Closed
		Limit Switch, Normally Closed, Held Open
		Temperature Actuated Switch, Opens On Rise
	<u> </u>	Temperature Actuated Switch, Closes On Rise
	⊸್ಷ⊸	Vacuum Switch, Opens On Rise
	~ ~ ~	Vacuum Switch, Closes On Rise
		Flow Switch (Closes With Flow)
	~T	Flow Switch (Opens With Flow)
	-√-	Float Operated Switch, Opens On Rise
	°	Float Operated Switch, Closes On Rise
		Torque Switch (Opens On Increase) Torque Switch (Closes On Increase)
	-x-	Overlaad
	*	Located Remote
	•	Located At Motor
	0	New Device To Be Provided
		Located At Unitized Control Panel (UCP)
	Δ	Located At Process Control Panel
		Motorized Time Delay Relay
		Time Delay Relay
	_ <u>_</u>	Starter Coil
	-R-	Control Relay
	—ETM—	Elapsed Time Meter
		Electric Damper Motor
	—DS—	Ductstat
	—AT)—	Adjustable Timer
ı	1	

SINC	GLE LINE DIAGRAMS
SYMBOLS	DESCRIPTION
	Voltmeter
(A)	Ammeter
vs	Voltmeter Switch
AS	Ammeter Switch
мм	Microprocessor Metering Unit
PFR	Phase Fail Relay
WT	Watts Transmitter (Power)
VFC	Variable Frequency Controller
	Transformer, Power Or Control
	Transformer, Grounded
$\stackrel{\perp}{\uparrow}$	Capacitor (Three Phase)
• •	Surge Arrestors (Three)
\downarrow	Neutral, Motor Or Generator
(PF)	Power Factor Meter
(T)	Temperature Meter
(A)	Voltammeter
W H	Watthour Meter
(WH)	Watthour Demand Meter
S	Instrument Transfer Switch
(GF)	Ground Fault Relay
K	Key Interlock
15)	Thermal Magnetic Circuit Breaker Or Motor Circuit Protector (Number Indicates Trip Ratio)
15 🖺	Fuse (Number Indicates Ampere Rating)
3—GF	Zonal Grounding
<u> </u> /-E	Switch As Specified (E Indicates Electric Operator)
—15-II—	Molded Case Circuit "Breaker". Or Motor Circuit Protector, Magnetic Storier, Control Transform, Aux. Contacts, Etc. As Specified (number indicates Trip Rating). T° Indicates Forward And T° Indicates Reverse Contactor.
15	Fused Switch Magnetic Starter; Control Transformer, Aux. Contacts, Etc. As Specified (Number Indicates Ampere Rating)
386.5	Current Transformer (CT) (Subscript Indicates Quantity And Ratio)
£ 480 /120	Current Transformer, Ring Or Doughnut Type. (Number Indicates Ratio)
480/120 	Potential Transformer (Number Indicates Ratio)
52	Circuit Breaker (Over 600V.)
	Fuse And Contactor (Over 600V.) With Disconnect Provision
*	Separable Connectors
4	Cable Terminators
Ø	Indicating Light
~777~	Space Heater
RTD	Resistance Temperature Detector
	Electric Motor — "Number" Indicates Approximate Horsepower, "F" Denotes Fractional Horsepower.
	·

<u>IBOLS</u>	DESCRIPTION	*	Miscellaneous Control, Metering And
\bigcirc	Voltmeter		Instrumentation Devices
(A)	Ammeter	*	Replace The Asterisk With One Of The Following Letters
\sim			Following Letters
VS	Voltmeter Switch		Synchronizing Check Undervoltage
AS	Ammeter Switch	32 -	Reverse Power
ММ	Microprocessor Metering Unit		Phase Sequence Thermal
PFR	Phase Fail Relay	50 -	Instantaneous Overcurrent
WT	Watts Transmitter (Power)		AC Time Overcurrent AC Power Circuit Breaker
			Power Factor Voltage Or Current Balance
VFC	Variable Frequency Controller	62 -	Time Delay
	Transformer, Power Or Control		Ground Directional Overcurrent
uiu		l	Lockout
\sim	Transformer, Grounded		Differential Current Transfer Trip
· · · ·		AM	Ammeter
<u></u>	Capacitor (Three Phase)	AH C	Amper-Hour Meter Coulombmeter
/ `		CMA	Contact—Making (Or Breaking) Ammeter
⊸ ⊶	Surge Arrestors (Three)	CMC	Contact-Making (Or Breaking) Clock
111	Neutral, Motor Or Generator	CRO	Contact—Making (Or Breaking) Voltmeter Oscilloscope Cathode—Ray Oscillograph
		CS DT	Breaker Control Switch Duty Transfer Switch
₽ F	Power Factor Meter	DB	Db (Decibel) Meter Audio Level/Meter
T	Temperature Meter	DBM	Dbm (Decidels Referred To 1 Milliwatt) Meter
(VA)	Voltammeter	DBX	Dead Bus Auxiliary Demand Meter
W)	Watthour Meter	DTR	Demand—Totalizing Relay
_		F FPR	Frequency Meter Feeder Protect Relay
WAD	Watthour Demand Meter	G	Device In Ground Circuit
S	Instrument Transfer Switch	GD GSR	Ground Detector
(GF)	Ground Fault Relay	IR	Ground Sensing Relay Interposing
K	Key Interlock	1	Indicating Meter
Ļ	Thermal Magnetic Circuit Breaker Or Motor	INT K	Integrating Meter Key Interlock
15)	Circuit Protector (Number Indicates Trip Ratio)	LOR	Lockout Relay
15 🖺	Fuse (Number Indicates Ampere Rating)	UA MM	Microammeter Microprocessor Metering
-	, , , , , , , , , , , , , , , , , , , ,	ММА	Microprocessor Metering And Analyzer
}—(GF)	Zonal Grounding	MR MS	Microprocessor Relay Metering System
, .	-	MA	Milliammeter
'/©	Switch As Specified (E Indicates Electric Operator)	N NM	Device In Neutral Circuit
ı	Molded Case Circuit "Breaker". Or Motor	OHM	Noise Meter Ohmmeter
 15 —	Circuit Protector, Magnetic Starter, Control	OP	Oil Pressure Meter
	Transformer, Aux. Contacts, Etc. As Specified (number Indicates Trip Rating). "F" Indicates Forward And "R" Indicates Reverse Contactor.	OSCG PF	Oscillograph, String Power Factor Meter
15 15	Fused Switch Magnetic Starter; Control Transformer, Aux. Contacts, Etc. As	PFR	Phase Fail Relay
15	Transformer, Aux. Contacts, Etc. As Specified (Number Indicates Ampere Rating)	PH PI	Phasemeter Position Indicator
#	Current Transformer (CT) (Subscript Indicates	PSR	Phase Sensing Relay
₹ +	Quantity And Ratio)	RD REC	Recording Demand Meter Recording Meter
#	Current Transformer, Ring Or Doughnut Type.	RF	Reactive Factor Meter
\$ 4#	(Number Indicates Ratio)	SY SS	Synchroscope Selector Switch
480/120		TM	Elapsed Time Meter
$\dashv \vdash$	Potential Transformer (Number Indicates Ratio)	TMR T	Timer Temperature Meter
52	Circuit Breaker (Over 600V.)	THC	Thermal Converter
52	Official diseases (over coort.)	TLM TS	Telemeter
*		п	Test Switches Total Time Meter,
首	Fuse And Contactor (Over 600V.) With Disconnect		Elapsed Time Meter
-	Provision	VM VA	Voltmeter Volt-Ammeter
¥		VAR	Varmeter
j	Secondary Communication	VARH VI	Varhour Meter Volume Indicator
个	Separable Connectors		Audio-Level Meter
\downarrow	Cable Terminators	VU VU	Standard Volume Indicator Audio—Level Meter
<u></u> ★ 日 ★ ★ ★ ★ ★ ★ ★ ★ ★ ★	Indicating Light	w	Wattmeter
 		WH WHD	Watthour Meter Watthour Demand Meter
	Space Heater	wt	Watts Transducer
RTD	Resistance Temperature Detector	×	Auxiliary
(5)	Electric Motor — "Number" Indicates Approximate Horsepower, "F" Denotes Fractional Horsepower.		



METROPOLITAN WATER RECLAMATION DISTRICT
OF GREATER CHICAGO
signed by: | Checked by: | Correct: | Approved: CTE AECOM

DISINFECTION FACILITIES ELECTRICAL LEGEND

CONTRACT 07-026-2P NORTH SIDE WATER RECLAMATION PLANT ULTRAVIOLET DISINFECTION FACILITIES

Sheet Number: E-001 Seal Page Number: 40

A	Ampere
AC	Alternating Current
ACC	AC Controller
AF	Circuit Breaker Frame Size (Amperes)
AFF	Above Finished Floor
AHU	Air Handling Unit
AIC	Ampere Interrupting Capacity
AM	Ammeter
AP	Auxiliary Panelboard
AS	Ammeter Switch
AT	Circuit Breaker Trip Rating (Amperes)
ATS	Automatic Transfer Switch
AWG	American Wire Gage
В	Boiler
BAS	Building Automation System
BATT	Battery Or Batteries
BFP	Boiler Feed Pump
BP	By Pass
BKR	Breaker Or Breakers
С	Conduit
CB	Circuit Breaker
CCTV	Closed Circuit Television
CEC	Chicago Electrical Code
CKT	Circuit
CLG	Ceiling
COMED	Commonwealth Edison Company
CP	Control Panel
СТ	Current Transformer
CTR	Contactor
DC	Direct Current
DCC	DC Controller
DCL	DC Link
DCS	Data Control System
DEH	Dehumidifier
DP	Distribution Panelboard
DN	Down
DP	Distribution Panel
DS	Disconnect Switch
DSS	Door Security System
DT	Day Tank
DWG	Drawing
E	Existing To Remain
EF	Exhaust Fan
EG	Equipment Ground
EL	Elevation
ELEC	Electrical
ELEV	Elevator
ELP	Emergency Lighting Panelboard
EM	Emergency
EMT	Electrical Metallic Tubing
EO	Electrically Operated
EQUIP	Equipment
ER	Existing To Be Relocated
EUH	Electric Unit Heater
EWC	Electric Water Cooler
EWH	Electric Water Heater
EXIST	Existing
EXPL	Explosion Proof

E	ELECTRICAL ABBREVIATIONS (CONT.)
F	Fuse Or Fuses
FAPP	Field Application Panel
FCP	Fire Alarm Control Panel
FDF	Forced Draft Fan
FDR	Feeder
FLUOR	Fluorescent
FO FRE	Fiber Optic Fiberglass Reinforced Epoxy
G	Ground Wire
GBC	Generator Battery Charger
GC	Generator Controller
GEN	Generator
GFI	Ground Fault Circuit Interrupter
GFF	Gas Fired Furnace
GMPC	Generator Master Paralleling Controller
GPC	Generator Paralleling Controller
GRD	Ground
GRS	Galvanized Rigid Steel
GSB	Generator Starting Battery
GSLC	Generator Switchgear Logic Controller
HP	Horsepower
HTR	Heater
HZ	Hertz
IDF	Induced Draft Fan
IG	Isolated Ground
INCAND	Incandescent
IP	Instrument Panelboard
1/0	Input/Output
IT	Input Transformer
JB	Junction Box
K	Kirk Key Interlock
kcmil	One Thousand Circular Mils
KV	Kilovolt
KVA	Kilovolt-Ampere
KW	Kilowatt
L	Load
LB	Load Bank
LC	Lighting Contactor
LOS	Lock-Out-Stop
LP	Lighting Panelboard
LS	Limit Switch
LTG	Lighting
М	Meter
MC	Momentary Contact
MF	Motor Field
MM	Microprocessor Metering
MMA	Microprocessor Metering And Analyzer
MCB	Main Circuit Breaker
MCC	Motor Control Center
MLO	Main Lug Only
MSLC	Main Switchgear Logic Controller
MSC	Main Switchgear Console
MTD	Mounted
MTG	Mounting
MTS	Manual Transfer Switch
MTU	Master Telemetry Unit
MVA	Million Volt Ampere

	LECTRICAL ABBREVIATIONS (CONT.)
N	Normal Source
NC	Normally Closed
NEC	National Electrical Code
NEMA	National Electrical Manufacturers Association
NEUT	Neutral
NO	Normally Open
NTS	Not To Scale
OBPC	Output/By-Pass Contactor
OL	Overload
Р	Pole
PA	Public Address
PB	Pullbox
PC	Photocell
PF	Power Factor
PGRS	Polyvinyl Chloride Coated Galvanized Rigid Steel
PH	Phase
PLC	Programmable Logic Controller
PM	Pump Motor
PNL	Panel
PP	Power Panelboard
PRI	Primary
PT	Potential Transformer
PVC	Polyvinyl Chloride
PWR	Power
REF	Return/Exhaust Fan
REC	Receptacle
RGS	Rigid Galvanized Steel
RP	Receptacle Panelboard
RR	Existing In Relocated Position
RTU	Remote Telemetry Unit
S	Speaker
SAP	Security Alarm Panel
SB	Stand-By
SBC	Station Battery Charger
SCADA	Supervisory Control And Data Acquisition
SCP	Steam Condensate Pump
SEC	Secondary
SF	Supply Fan
SMP	Sump Pump
SP	Supplemental Panelboard
SS	Stainless Steel
ST	Shunt Trip
STCP	Storage Tank Control Panel
SW	Switch
SWGR	Switchgear
Т	Transformer
TEL	Telephone
TB	Termination Box With Termination Strips
TDC	Time Delay Contact
TDR	Time Delay Relay
TEF	Toilet Exhaust Fan
TTB	Telephone Terminal Board
ттс	Telephone Terminal Cabinet
110	<u> </u>
TYP	Typical

	ELECTRICAL ABBREVIATIONS (CONT.)								
UH Unit Heater									
UL	Underwriters Laboratories, Inc.								
UNO	Unless Noted Otherwise								
UPS	Uninterruptible Power System								
USS	USS Unit Secondary Substation								
UTP	UTP Unshielded Twisted Pair								
V	Volt								
VA	Volt Ampere Variable Frequency Controller								
VFC									
VFD	Variable Frequency Drive								
VFDC	VFDC Variable Frequency Drive Controller								
VM	Voltmeter								
VS	Voltmeter Switch								
W	Wire								
(W)	Watt								
WHM	Watthour Meter								
WP	Weatherproof Device								
X	Existing To Be Removed								

ELECTRICAL EQUIPMENT LEGEND									
ATS-XX1	Automatic Transfer Switch								
AHU-XX1	Air Handling Unit								
B-XX1	Boiler								
BFP-XX1	Boiler Feed Pump								
DP-XX1	Distribution Panelboard								
DS-XX1	Disconnect Switch								
EF-XX1	Exhaust Fan								
ELP-XX1	Emergency Lighting Panelboard								
EUH-XX1	Electric Unit Heater								
EWC-XX1	Electric Water Cooler								
EWH-XX1	Electric Water Heater								
GEN-XX1	Generator								
GFF-XX1	Gas Fired Furnace								
GCPR-XX1	Gas Compressor								
IDF-XX1	Boiler Induced Draft Fan								
LP-XX1	Lighting Panelboard								
MCC-XX1	Motor Control Center								
PACE-XX	Public Address Control Equipment								
PM-XX1	Pump Motor								
PP-XX1	Power Panelboard								
REF-XX1	Return/Exhaust Fan								
RP-XX1	Receptacle Panelboard								
SCP-XX1	Steam Condensate Pump								
SF-XX1	Supply Fan								
SP-XX1	Sump Pump								
SWGR-XX1	Switchgear								
T-XX1	Transformer								
TEF-XX1	Toilet Exhaust Fan								
UH-XX1	Unit Heater								
USS-XX1	Unit Secondary Substation								
VFD-XX1	Variable Frequency Drive								
VS-XX	Conduit Schedule								
XX = EM = LB = UPS = SL =	EM = Emergéncy LB = Load Bank UPS = Uninterruptible Power SL = Stand-By System, Lighting SP = Stand-By System, Power								



CONTRACT 07—026—2P

NORTH SIDE WATER RECLAMATION PLANT

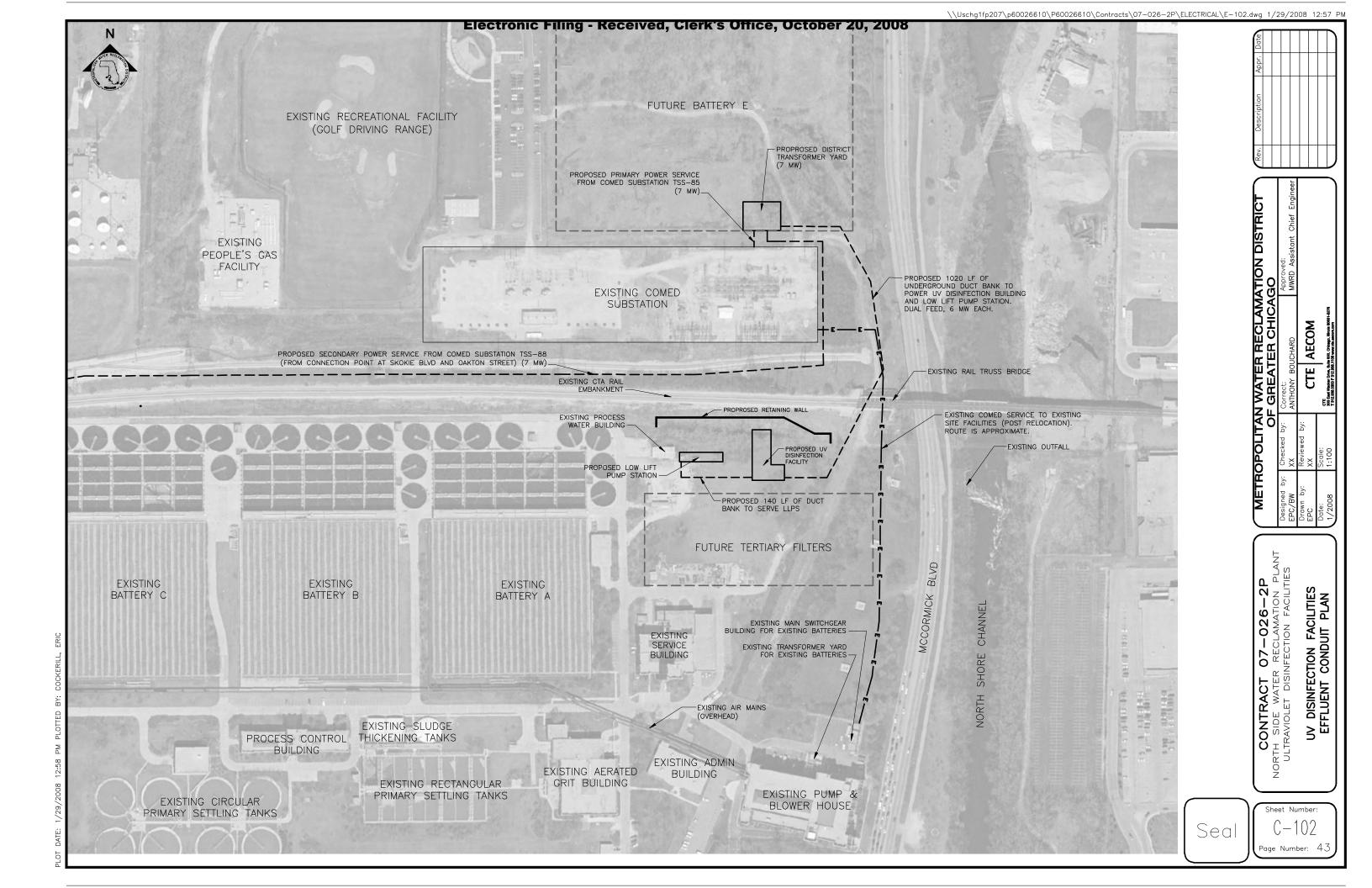
ULTRAVIOLET DISINFECTION FACILITIES

UV DISINFECTION FACILITIES

ELECTRICAL ABBREVIATIONS

Seal

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Seal

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ſ		<u>PUMPS</u>	BLO	<u>DWERS</u>	electroni EC	C Filing - Received,	Clerk's	Office, October 2	20, 2008	VALVES	VALVE OPERATORS	
	\bigcirc	Centrifugal — Horizontal Pump	€ C	Centrifugal Blower	ı		\dashv	Blind Flange	M	Ball Valve	Pneumatic Operator (Cyli	inder)
	<u>≠</u>		\bigotimes			Paddle Mixer	Y	Drain	N	Butterfly Valve	Diaphragm	₹
		Centrifugal — Vertical Pump	R	Rotary Blower		Propeller Mixer	<u>©</u>	Clean Out	2	Check Valve — Swing		cription
		Diaphragm Pump	C	Centrifugal Compressor	<i>∞</i> ∞		-{} -	Diaphragm Seal Fire Hydrant	\Box	Cone Valve	Pressure Regulating Diapl	nragm 8
		Dauble Suekies Duese	R	Reciprocating Compressor		Turbine Mixer	Ω Υ	Flexible Connection			Differential Pressure Regulating Diaphragm	Rev.
		Double Suction Pump	<u>TA</u>	<u>INKS</u>		Centrifuge	├	Flexible Hose		Diaphragm Valve	Pressure Reducing Diaphr	ragm
		Inline Pump	S	Storage Tank — Vertical			- -	Quick Connect Coupling Hose Connection (Threaded)	\bowtie	Gate Valve	Motor	RICT
		Metering Pump		Storage Tank — Horizontal		Grit Classifier		Reducer — Concentric	D ≪ J T	Globe Valve	Solenoid	IST
				storage rank – monzontal	\rightarrow			Reducer — Eccentric		Knife Valve	E/H Hydraulic Operator	ON O
		Piston Pump	A	Aeration Tank		Compactor		Rubber Expansion Coupling Vent	\angle	Angle Valve		WATER RECLAMATION DISTRICT GREATER CHICAGO Forrect: NITHONY BOUCHARD CTE AECOM TITL AEC
		Positive Displacement Pump				Belt Conveyor	 	Y-Strainer	\boxtimes	3 Way Valve	VALVE NORMAL STATUS ✓ or NO Normally Open	IICAM
		rositive displacement rump	s	Sand Filter or Trickling Filter	<u></u>	S	⊗	Yard Hydrant		4 Way Valve	or NC Normally Closed	RECLES CHILLIAND
		Progressive Cavity Pump			f vvv Fl	Screw Conveyor	INSTRUMENT	S/FLOW DEVICES		+ may valve	LINE DESIGNATIONS	ER FATER
		Rotary/Gear Pump	FI	iat-top Tank/Reservoir	7	Bucket Elevator		Area Velocity Meter	\bowtie	Needle Valve	Service Abbreviation XX-YYY-ZZZ	WATE 3REA 1Rect: HONY BG CTE
								Area verderty weter	7	Backflow Preventer	Line Size (Inches) See Note 2. Material Abbreviation	OF GO CONTRACT
		Screw Pump		Storage Reservoir or Digester — Fixed Cover		Belt Filter Press		Averaging Pitot	\bowtie	Pinch Valve	EQUIPMENT TAGS	
		Sump Pump	D	Digester — Floating Cover	*			Flow Nozzle	\bowtie	Plug Valve	ldentifiction Numb	TROPOLL TO Scale: NY: Checked It XX XX Scale: NOT TO SC
				Clarifier/Thickener	*	Sludge Grinder		Float Type Level Sensor	-	Pressure/Air Relief Valve — Angl		sor L pe so so
		Submersible Pump		Jamiery mickener		Heat Exchanger — Spiral	M	Magmeter	- <u>*</u>	Vacuum Breaker Valve (Air Inlet)	B - Blower ET - Elevated V - Valve T - Tank SG - Slide/Weir Gate MX - Mixer UVR - UV Reactor D - Digester	Dote: 1/2006
	Î L		s	Spheroidal Elevated Tank		Heat Exchanger — Plate and Frame		Parshall Flume		Pressure Relief Valve — Straight	UVM — UV Module S — Screen NOTES:	Ę
		Vertical Axial Flow Pump				Sampler	Image: section of the content of the	Pitot Tube	-	Combination Air/Vacuum Valve	Symbology based on Instrument, S Automation Society (ISA) Standard except as modified hereing.	.: ILL □ □ I
						Inline Static Mixer	8	Propeller/Turbine Meter		<u>GATES</u>	Rectangular channel sizes given c X'-Y"xX'-Y"	
. ERIC	<u> </u>	Vertical Turbine Pump				Bar Screen	\bowtie	Restrict Orifice	\mathbb{I}	Slide Gate	Valves shown without operators s assumed to include local manual operators.	Photos of library and library in the No. 1 FAC ON LE
OCKERILL		Archimedes Screw Pump	MISCE	<u>ELLANEOUS</u>		Fine Screen		Rotameter	口 + 山	Stop Plate/Logs	 P&IDs are not intended to show details of piping, joints, supports, Contractor shall install a complete 	etc. e system
ID BY: C		Peristaltic Pump/Hose Pump		Flow (DWG Continuation) Interlock		Gravity Belt Thickener	TM-	Thermal Mass Flowmeter	I	Weir Gate	per the Contract Documents and required to provide a fully functio system.	TRAC E WAT DISINF
PLOTTE			\$ H/0/A S	Selector Switch			\approx	Trapezoidal Flume				S S S S S S S S S S S S S S S S S S S
0:50 AM			[Roll—off Container		Open Channel UV Reactor (Med Pres	s) 🔾	Ultrasonic Level Sensor				ORT
2008 1			F	Hopper				Venturi				
1/17/:				Liquid or Sludge Hauling Truck				V-Notch Weir				Sheet Number:
OT DATE:				Dump Truck								$\begin{bmatrix} -001 \\ Page Number: 46 \end{bmatrix}$

							TYPIC	AL LETT	ER COM	BINATIO	NS									
FIRST	INITIATING OR MEASURED		CONTR	OLLERS		READOU [*]	T DEVICES	SWITCHES	AND ALARM	DEVICES ¹	1	TRANSMITTER	S	SOLENOIDS, RELAYS,	PRIMARY	TEST	WELL OR	VIEWING DEVICE,	SAFETY	FINAL
LETTERS	VARIABLE	RECORDING	INDICATING	BLIND	SELF-ACTUATED CONTROL VALVES	RECORDING	INDICATING ²	HIGH	LOW	сомв.	RECORDING	INDICATING	BLIND	COMPUTING DEVICES		POINT	PROBE	GLASS	DEVICE	ELEMENT
Α	ANALYSIS	ARC	AIC	AC		AR	Al	ASH	ASL	ASHL	ART	AIT	AT	AY	AE	AP	AW			AV
В	BURNER/ COMBUSTION	BRC	BIC	BC		BR	ВІ	BSH	BSL	BSHL	BRT	BIT	ВТ	BY	BE		BW	BG		BZ
С	USER'S CHOICE																			
D	USER'S CHOICE																			
E	VOLTAGE	ERC	EIC	EC		ER	EI	ESH	ESL	ESHL	ERT	EIT	ET	EY	EE					EZ
F	FLOW RATE	FRC	FIC	FC	FCV, FICV	FR	FI	FSH	FSL	FSHL	FRT	FIT	FT	FY	FE	FP		FG		FV
FQ	FLOW QUANTITY	FQRC	FQIC			FQR	FQI	FQSH	FQSL			FQIT	FQT	FQY	FQE					FQV
FF	FLOW RATIO	FFRC	FFIC	FFC		FFR	FFI	FFSH	FFSL						FE		+			FFV
G	USER'S CHOICE		1110	110						110										1111
H	HAND CURRENT	IRC	HIC	нс		IR	1 11	ISH	ISL	HS ISHL	IRT	IIT	IT	IY	IE		-			HV IZ
J	POWER	JRC	JIC			JR	JI	JSH	JSL	JSHL	JRT	JIT	JT	JY	JE		-			JV
K	TIME	KRC	KIC	KC	KCV	KR KR	KI	KSH	KSL	KSHL	KRT	KIT	KT	KY	KE		+			KV
L	LEVEL	LRC	LIC	LC	LCV	LR	LI	LSH	LSL	LSHL	LRT	LIT	LT	LY	LE		LW	LG		LV
M	USER'S CHOICE	LNC	LIC	LC	LCV	LK	LI	LON	LSL	LONE	LKI	LII	LI	LI	LE		LVV	LG		+ Lv
N	USER'S CHOICE										+									
0	USER'S CHOICE																			+
	PRESSURE/																+		PSV,	+
Р	VACUUM [*]	PRC	PIC	PC	PCV	PR	PI	PSH	PSL	PSHL	PRT	PIT	PT	PY	PE	PP			PSE PSE	PV
PD	PRESSURE, DIFFERENTIAL	PDRC	PDIC	PDC	PDCV	PDR	PDI	PDSH	PDSL		PDRT	PDIT	PDT	PDY	PE	PP				PDV
Q	QUANTITY	QRC	QIC			QR	QI	QSH	QSL	QSHL	QRT	QIT	QT	QY	QE					QZ
R	RADIATION	RRC	RIC	RC		RR	RI	RSH	RSL	RSHL	RRT	RIT	RT	RY	RE		RW			RZ
S	SPEED/ FREQUENCY	SRC	SIC	SC	SCV	SR	SI	SSH	SSL	SSHL	SRT	SIT	ST	SY	SE					SV
Т	TEMPERATURE	TRC	TIC	TC	TCV	TR	TI	TSH	TSL	TSHL	TRT	TIT	TT	TY	TE	TP	TW		TSE	TV
TD	TEMPERATURE, DIFFERENTIAL	TDRC	TDIC	TDC	TDCV	TDR	TDI	TDSH	TDSL		TDRT	TDIT	TDT	TDY	TE	TP	TW			TDV
U	MULTIVARIABLE					UR	UI							UY						UV
٧	VIBRATION/ MACHINERY ANALYSIS					VR	VI	VSH	VSL	VSHL	VRT	VIT	VT	VY	VE					VZ
w	WEIGHT/ FORCE	WRC	WIC	WC	WCV	WR	WI	WSH	WSL	WSHL	WRT	WIT	WT	WY	WE					wz
WD	WEIGHT/FORCE, DIFFERENTIAL	WDRC	WDIC	WDC	WDCV	WDR	WDI	WDSH	WDSL		WDRT	WDIT	WDT	WDY	WE					WDZ
Х	UNCLASSIFIED																			
Y	EVENT/STATE/ PRESENCE		YIC	YC		YR	YI	YSH	YSL				YT	YY	YE					YZ
Z	POSITION/ DIMENSION	ZRC	ZIC	ZC	ZCV	ZR	ZI	ZSH	ZSL	ZSHL	ZRT	ZIT	ZT	ZY	ZE					ZV
ZD	GAUGING/ DEVIATION	ZDRC	ZDIC	ZDC	ZDCV	ZDR	ZDI	ZDSH	ZDSL		ZDRT	ZDIT	ZDT	ZDY	ZDE	AP	AW			ZDV

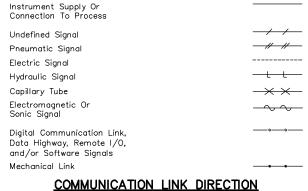
² "I" for Indicator can be substituted with "L" for Pilot Lights

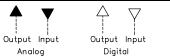
OTHER POSSIBILE COMBINATIONS

FO	(Restriction Orifice)	PFR	(Ratio)
FRK, HIK	(Control Stations)	KQI	(Running Time Indicator)
FX	(Accessories)	QQI	(Indicating Counter)
TJR	(Scanning Recorder)	WKIC	(Rate-Of-Weight-Loss Controller)
LLH	(Pilot Light)	HMS	(Hand Momentary Switch)

INSTRUMENT LINE SYMBOLS

All Lines To Be Fine In Relation To Process Piping





INSTRUMENT DESIGNATIONS

XXX — Control Variable (See Tabl	le Above)
YYY - Location, Service, and Poi	int Number
Z - Optional Function Character	as follows:
A — Start	P — Remote
B - Stop	Q — High Torque Alarm
C - Close	R – Run
E - Emergency Stopped	S — Select
F — Fail	T — High Temperature Al
G — Run Forward	U — In Automatic
J — Start Forward	W - Jog Reverse
K - Start Reverse	X - Run Reverse
L — Leak Detection	Y - Full Opened
	7 5 11 01 1

M — Motion Failure 0 - Open

NOTES:

- The Process And Instrumentation Diagram (P&ID) Drawings Include Field Mounted And Primary Location Devices Which Serve As Input/Output I/O) Points To The Programmable Logic Controller (PLC). The P&ID's Do Not Depict All Local Control Stations And Local Control Panel Components. The Items Depicted Represent The Minimum Requirements Of The I/O Points To Be Integrated Into The System And To Be Displayed On The Process Instrumentation And Control System (PICS) Operator Interface. Refer To Specification Section 13304— System Control For Local Control Station Components, Control Panel Components, Operational Description, Etc. All I/O Shall Be Obtained From The Equipment Manufacturers Control Panel. Provide And Install Auxilliary Relays And Contacts As Required To Obtain I/O To Be Integrated Into The PLC.
- 2. Refer To Specification Section 13390 Input/output (I/O) Point List For Listing Of Additional I/O Points.
- 3. The Pilot Light Symbol Is Used To Depict Event/state/presence Conditions That Are To Be Displayed On The PICS Operator Interface In Addition To The Same Conditions Displayed Locally At The Local Control Station Or Local Control Panel.

 Refer To Specification Section 13304 — System Control For Local Control Station And Local Control Panel Requirements.

INSTRUMENT MODIFIERS

	HOR L/R Local/Remote LOP Local/Off/PLC MA Manual/Auto MOA On/Off/Auto OOA On/Off/Auto OOP OOP OOP OOP OOP OOP Sys Start/Stop/Auto
--	---

GENERAL INSTRUMENT OR FUNCTION SYMBOLS

	PRIMARY LOCATION NORMALLY ACCESSIBLE TO OPERATOR	FIELD MOUNTED	AUXILIARY LOCATION NORMALLY ACCESSIBLE TO OPERATOR
DISCRETE INSTRUMENTS	\ominus		Θ
SHARED DISPLAY, SHARED CONTROL			
COMPUTER FUNCTION			
PROGRAMMABLE LOGIC CONTROL			
PILOT LIGHT (SEE NOTE NO.3)	X		



METR	DPOLITA	METROPOLITAN WATER RECLAMATION DISTRICT	ATION DISTRICT
	ō	OF GREATER CHICAGO	(GO
Designed by:	Designed by: Checked by: Correct:	Correct:	Approved:
EPC	××	ANTHONY BOUCHARD	MWRD Assistant Chief Engineer
Drawn by:	Reviewed by:		
EPC	××	CTE AECOM	
Date:	Scale:		
1/2008	NOT TO SCALE	NOT TO SCALE 803 East Wardow Drive, Suits 600, Chicago, Illinois 60601-5278	

CONTRACT 07-026-2P NORTH SIDE WATER RECLAMATION PLANT ULTRAVIOLET DISINFECTION FACILITIES UV DISINFECTION FACILITIES INSTRUMENTATION ABBREVIATIONS

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